

CITY OF BURNET ENGINEERING

ADDENDUM NO. 4

November 27, 2023

Burnet City Hall – RFP 2023-012 PID: CIPSP-2022B

Ladies/Gentlemen:

The City of Burnet issues this Addendum on the above captioned project. This Addendum details the changes and the respective bid document pages which were added and/or changed. Each bidder is required to acknowledge receipt of this Addendum, on the form included in the Addendum below. Failure to acknowledge receipt of this Addendum in your bid proposal will result in your bid not being read.

Please sign and <u>return</u> this Addendum with your sealed bid packet at time and location as advertised on the Invitation to Bid for this project. Addendum should be located at the beginning of the packet. Failure to acknowledge all the addenda issued will result in bid dismissal.

DESIGN ENGINEER APPROVAL:

Eric Belaj, PE, CFM
City Engineer

Addendum items and pertaining attachment will be included in the following pages.

BIDDER ADDENDUM ACKNOWLEDMENT BELOW:

(Company Name)

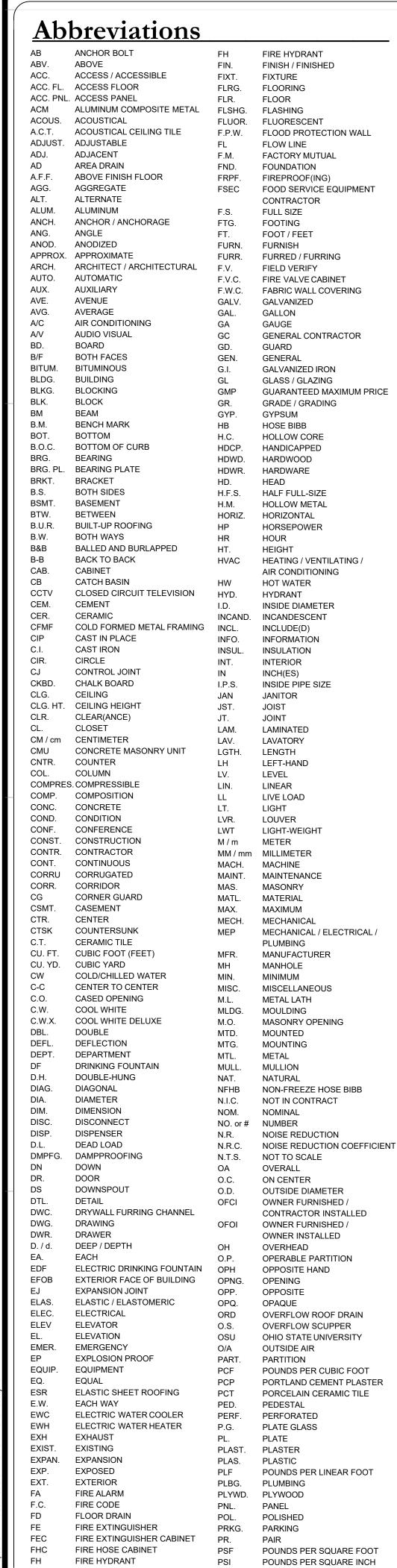
(Bidder Rep. Name)

By: _______ Date: _______
(Signature)



CITY OF BURNET ENGINEERING

- 1. Bonds: No Change
- 2. **Dates:** No Change. See previous Addendums 1-3.
- 3. **Questions and Answers**: This Bid Addendum also answers questions posed by contractors throughout the bid process and at the pre-bid meeting as follows:
 - a. Question from Bidder. See CIVAST
 - a. Answer. See CivCast
- 4. **<u>Bid Document Changes</u>**: This item outlines changes to the Bid Documents:
 - a) Revisions to the structural foundation design to add piers to the design in lieu of previous design.
 - b) The addendum clarifies the discrepancy between Geotechnical report and Structural Foundation design as noted below:
 - Existing soil must be removed under the building area or other structures as noted in the updated Sheet No. S0.01.
 - Drilled piers shall be installed under the foundation area as noted in the attached design.
 - c) See attached updated MEP drawing M2.14 clarifying testing requirements and installation standards, and other changes as noted in the design and attached specification.



STOR.

SUSP.

STRUCT.

STORAGE

SUSPENDED

SYNTHETIC

SYSTEM

SYMMETRICAL

SUPPLY AIR DIFFUSER

STRUCTURE / STRUCTURAL



PLUS OR MINUS Graphic Symbols EXTERIOR ELEVATION NORTH ARROW **BUILDING SECTION** SECTION DETAIL DETAIL REFERENCE REVISION NUMBER AND SHFFT NUMBER \ AFFECTED AREA **DETAIL NUMBER** INTERIOR ELEVATION SHEET NUMBER -9'-5 57/64" VERTICAL ELEVATION ELEVATION NUMBER **ROOM NAME** ROOM IDENTIFICATION ROOM NUMBER SEMI-RECESED FIRE EXTINGUISHER VERTICAL ELEVATION ACCESS CONTRO EXISTING ELEVATION CARD READER NEW ELEVATION PARTITION TYPE (RE: PARTITION SCHEDULE FOR DESCRIPTION) DOOR NUMBER (RE: DOOR SCHEDULE) WINDOW TAG AP4 APPLIANCE TAG (RE: APPLIANCE SCHEDULE) PLUMBING TAG (RE: PLUMBING SCHEDULE) MATCH LINE COLUMN GRID INDENTIFICATION — † — A EXISTING SPEC. SECTION KEYNOTE LETTER SUFFIX

THE KEYNOTING SYSTEM USED ON THESE DRAWINGS IS FOR MATERIALS REFERENCES AND NOTES. REFER TO THIS LEGEND FOR INFORMATION THAT RELATES TO EACH KEYNOTE SYMBOL ON THI DRAWINGS. EACH KEYNOTE SYMBOL CONSISTS OF A 5-DIGIT NUMBER, THAT RELATES TO THE SPECIFICATION SECTION, WHICH GENERALLY COVERS THE ITEM, THAT IS REFERENCED, AND A LETTER SUFFIX. THE LETTER SUFFIX DOES NOT RELATE TO ANY CORRESPONDING REFERENCE LETTER IN THE

SPECIFICATION. THE ORGANIZATION OF THE KEYNOTING SYSTEM ON THESE DRAWINGS, WITH THE KEYNOTE REFERENCE NUMBERS RELATED TO THE SPECIFICATIONS SECTIONS NUMBERING SYSTEM. SHALL NOT CONTROL THE CONTRACTOR IN DIVIDING THE WORK AMONG SUBCONTRACTORS OR IN ESTABLISHING THE EXTENT OF WORK TO BE PERFORMED BY ANY TRADE.



ARCHITECT OF RECORD

SEAUX - PIERCE Architecture

1014 SAILMASTER ST.

(512) 419-9301

MEP ENGINEER

ATUSTIN, TEXAS 78734

Project Data & Code Analysis

TDLR #: TABS2023020006

BUILDING CODE CRITERIA

PLATE

ANGLE

PER

SQUARE

JURISDICTION: BURNET TEXAS

Vicinity Map

BUILDING CODE: (IBC) 2015 INT. BUILDING CODE ELECTRIC CODE: (IEC) 2014 NATIONAL ELECTRIC CODE MECH. CODE: INT. MECH. CODE (IMC) 2015 PLUMBING CODE: PLUMBING CODE (IPC) 2015 INT. ENERGY CONS.CODE **ENERGY CODE:** (IECC) 2015 FIRE CODE: INT. FIRE CODE (IFC) 2015 FUEL GAS CODE: INT. FUEL GAS CODE (IFGC) 2015

SITE ADDRESS: 301 East Jackson Street Burnet Texas 78611 USA

ZONING: **G-GOVERNMENT / PUBLIC**

OCCUPANCY: (AREA ALLOWANCES PER 2015 IBC, TABLE 1004.1.2) SEE SHEET G.4 LIFE SAFETY SHEET FOR EACH AREA.

> **B-BUSINESS** = 114 OCCUPANTS ACCESSORY AREAS = 4 OCCUPANTS 333 OCCUPANTS TOTAL

CONSTRUCTION TYPE: V-A

SPRINKLED: YES

BUILDING AREA: THE FOLLOWING SQUARE FOOTAGE IS TO THE INSIDE FACE OF STUD.

CONDITIONED: BUILDING 16,709 S.F. NON-CONDITIONED: COVERED PORCHES 543 S.F. TOTAL AREA 543 S.F.

PLAZA AND ELEVATED SIDEWALK 13,119 S.F.

EXITS: TWO EXITS REQUIRED WHEN OCCUPANT LOAD IS GREATER THAN 50 OR TRAVEL DISTANCE IS MORE THAN 75' PLUMBING: **B-BUSINESS** = 114 OCCUPANTS /2 = 57

WATER CLOSETS: 1 PER 25 FOR FIRST 50 AND 1 PER 50 THERAFTER REQUIRED W.C: PROVIDED W.C: **LAVATORIES**: 1 PER 40 FOR THE FIRST 80 AND 1 PER 80 THERAFTER REQUIRED:

PROVIDED: **A-3 ASSEMBLY** = 215 OCCUPANTS /2 = 107 WATER CLOSETS: 1 PER 125 FOR MALE & 1 PER 65 FOR FEMALE REQUIRED W.C:

PROVIDED W.O **LAVATORIES**: 1 PER 200 REQUIRED: PROVIDED: **DRINKING FOUNTAINS:** 1 PER 100

REQUIRED:

REQUIRED INSPECTIONS:

1. Temporary electric power inspection

PROVIDED: 1 (+BREAK ROOM) PARKING: 16,709 DIVIDED BY 250 SF OF BUILDING AREA = 66 REQUIRED

*Special Inspections Letters of Concurrence by designer must be turned into the city prior to Final

PROVIDED PARKING = ACCESSIBLE PARKING PROVIDED = 2 (1 VAN + 1 HC) TOTAL PARKING ADDITIONAL OFF-STREET PARKING ACROSS VANDERVEER STREET)

2. Water/ sewer yard line inspection 3. Underground electric inspection 4. Plumbing rough-in inspection 5. Water supply, Ufer ground embedded conduits inspection 6. *Foundation pre-pour inspection 7. *Special Inspections 8. Frame, Mechanical, Electric, and Plumbing rough-in inspection 9. Re-frame, Above ceiling and Energy 1 inspection 10. Gypboard inspection 11. Gas test inspection 12. Electric meter inspection 13. Final inspection (Building, Mechanical, Electric, Plumbing, and Energy 2)

Project Team Members

CITY OF BURNET 1001 BUCHANAN DRIVE BURNET, TX, 78611 P (512) 715-3208

STRUCTURAL ENGINEER FORT STRUCTURES 2120 EAST 7TH STREET, #200 **AUSTIN, TX 78702**

POWER FORWARD 4409 MERLE DRIVE AUSTIN, TX 78745 RICHARD@POWERFORWARDENG.COM (512)817-9264 SHANE@FORTSTRUCTURES.COM M: 512.956.2971

CIVIL ENGINEER CITY OF BURNET ATTN: ERIC BELAJ 1001 BUCHANAN DRIVE, SUITE 4

BURNET, TX 78611

(512) 715-3217

LANDSCAPE ARCHITECT

MPLA LANDSCAPE ARCHITECTURE 301 W. CREEK STREET FREDERICKSBURG, TX 78624 (830) 992-3710 ATTN: MARCUS PARKER, PLA

M0.00 MECHANICAL GENERAL NOTES

Index of Drawings ARCHITECTURAL

RCHITECT		ISSUED	REVISED
G0.1	PROJECT INFORMATION	11/10/2023	
G.1	TAS 01	9/15/2023	
G.2	TAS 02	9/15/2023	
G.3	CODE ANALYSIS	9/15/2023	
G.4	LIFE SAFETY SHEET (ADDED PER PLAN REVIEW)	11/10/2023	
A0.1	SITE PLAN	9/15/2023	
A0.2	SITE DETAILS	9/15/2023	
A0.3	SITE DETAILS	11/10/2023	
A1.1	P-TYPES	9/15/2023	
A1.2	DOOR TYPES SCHEDULES	11/10/2023	
A1.3	GLASS PARTITION TYPES SCHEDULES	9/15/2023	
A1.4	WINDOW OPENING TYPES SCHEDULES	11/10/2023	
A1.5	DOOR & WINDOW DETAILS	9/15/2023	
A2.1	FLOOR PLAN SHELL	9/15/2023	
A2.2	FLOOR PLAN	9/15/2023	
A2.3	ROOF PLAN	11/10/2023	
A2.4	ENLARGED FLOOR PLAN	11/10/2023	
A2.5	ROOM FINISH	11/10/2023	
A2.6	RCP	11/10/2023	
A3.1	EXTERIOR ELEVATIONS	11/10/2023	
A4.1	BUILDING SECTIONS	11/10/2023	
A4.2	WALL SECTIONS	11/10/2023	
A4.3	WALL SECTIONS	11/10/2023	
A4.4	PLAN DETAILS	11/10/2023	
A5.1	INTERIOR ELEVATIONS	9/15/2023	
A5.2	INTERIOR ELEVATIONS	9/15/2023	
A5.3	INTERIOR ELEVATIONS	9/15/2023	
A5.4	INTERIOR ELEVATIONS	9/15/2023	
A5.5	INTERIOR ELEVATIONS	9/15/2023	
A5.6	INTERIOR ELEVATIONS	9/15/2023	
A5.7	INTERIOR ELEVATIONS	9/15/2023	
A6.1	INTERIOR DETAILS	9/15/2023	
A6.2	INTERIOR DETAILS	11/10/2023	
A7.1	3D VIEWS	9/15/2023	
ANDSCAPE		ISSUED	REVISED
L1.1	LANDSCAPE MATERIALS PLAN	5/19/2023	
L1.2	LANDSCAPE MATERIALS PLAN ENLARGEMENT AT PLAZA	5/19/2023	
L2.1	LANDSCAPE LAYOUT PLAN	5/19/2023	
L2.2	LANDSCAPE LAYOUT PLAN ENLARGEMENT AT PLAZA	5/19/2023	
L3.1	LANDSCAPE PLANTING PLAN	5/19/2023	
L3.2	LANDSCAPE PLANTING PLAN ENLARGEMENT AT PLAZA	5/19/2023	
L4.1	DIAGRAMMATIC IRRIGATION PLAN	5/19/2023	
L4.2	DIAGRAMMATIC IRRIGATION PLAN ENLARGEMENT AT PLAZA	5/19/2023	
L5.1	FOUNTAIN PLAN OVERVIEW AT PLAZA	5/19/2023	
L5.2	FOUNTAIN PLANS AND SECTIONS	5/19/2023	
L5.3	FOUNTAIN - ELECTRICAL AND BONDING NOTES	5/19/2023	
RUCTUR	AL	ISSUED	REVISED
S0.01	STRUCTURAL GENERAL NOTES	11/14/2023	
S0.02	STRUCTURAL GENERAL NOTES	1.55	11/27/2023
S0.10	ABBREVIATIONS & LEGENDS	11/14/2023	11/27/2023
S1.01	AXONOMETRIC VIEWS	11/14/2023	0_0
S2.00	PIER LAYOUT PLAN	1 11 11 11 11	11/27/2023
S2.01	FOUNDATION PLAN	11/14/2023	11/27/2023
S2.02	ROOF FRAMING PLAN	11/14/2023	
S2.03	HIGH ROOF FRAMING PLAN	11/14/2023	
S3.01	BRACED FRAME ELEVATIONS	11/14/2023	
	BRACED FRAME DETAILS	11/14/2023	
	TYPICAL FOUNDATION DETAILS	11/14/2023	
	TYPICAL FOUNDATION DETAILS TYPICAL FOUNDATION DETAILS	11/14/2023	
-			11/27/2022
S4.10	FOUNDATION DETAILS FOUNDATION DETAILS - SITE RETAINING WALLS		11/27/2023
S4.11		11/14/2023	
S7.01 S7.02	TYPICAL BASE PLATE AND HSS COL. STEEL DETAILS	11/14/2023	
57 02	TYPICAL STEEL BEAM CONNECTION DETAILS	11/14/2023	
	TYPICAL POOF K OMO LETAL C		
S7.03	TYPICAL ROOF K OWSJ DETAILS	11/14/2023	
	TYPICAL ROOF K OWSJ DETAILS FRAMING DETAILS TYPICAL COLD-FORMED STEEL DETAILS	11/14/2023 11/14/2023 11/14/2023	

11/14/2023

S9.02 TYPICAL COLD-FORMED STEEL DETAILS

/12.11	MECHANICAL DUCTWORK PLAN - AREA A	5/19/2023	
12.12	MECHANICAL DUCTWORK PLAN - AREA B	7/21/2023	
/l2.13	MECHANICAL DUCTWORK PLAN - AREA C	5/19/2023	
12.14	MECHANICAL DUCTWORK PLAN - AREA D	5/19/2023	11/27/2023
12.21	MECHANICAL PIPING PLAN - AREA A	5/19/2023	, , ,
12.22	MECHANICAL PIPING PLAN - AREA B	7/21/2023	
12.23	MECHANICAL PIPING PLAN - AREA C	5/19/2023	
12.24	MECHANICAL PIPING PLAN - AREA D	5/19/2023	
13.00	MECHANICAL ROOF PLAN	5/19/2023	
15.00	MECHANICAL DETAILS	5/19/2023	
15.01	MECHANICAL DETAILS	6/30/2023	
16.00	MECHANICAL SCHEDULES	5/19/2023	
	MECHANICAL SCHEDULES	7/21/2023	
16.01 17.00			
17.00	MECHANICAL RISER DIAGRAMS - VRF-1	5/19/2023	
17.01	MECHANICAL RISER DIAGRAMS - VRF-2	5/19/2023	
0.00	PLUMBING GENERAL NOTES	5/19/2023	
21.01	PLUMBING SITE PLAN	6/30/2023	
2.11	PLUMBING PLAN - SANITARY DRAIN AND VENT - AREA A	5/19/2023	
2.12	PLUMBING PLAN - SANITARY DRAIN AND VENT - AREA B	7/21/2023	
2.13	PLUMBING PLAN - SANITARY DRAIN AND VENT - AREA C	5/19/2023	
2.14	PLUMBING PLAN - SANITARY DRAIN AND VENT - AREA D	5/19/2023	
2.21	PLUMBING PLAN - DOMESTIC WATER - AREA A	5/19/2023	
2.22	PLUMBING PLAN - DOMESTIC WATER - AREA B	7/21/2023	
2.23	PLUMBING PLAN - DOMESTIC WATER - AREA C	5/19/2023	
2.24	PLUMBING PLAN - DOMESTIC WATER - AREA D	5/19/2023	
25.00	PLUMBING DETAILS	5/19/2023	
25.01	PLUMBING DETAILS	6/30/2023	
6.00	PLUMBING SCHEDULES	7/21/2023	
7.00	PLUMBING RISER DIAGRAMS	7/21/2023	
0.00	ELECTRICAL GENERAL NOTES	5/19/2023	
1.01	ELECTRICAL SITE PLAN	6/30/2023	
1.02	ELECTRICAL SITE PHOTOMETRIC PLAN	5/19/2023	
2.11	ELECTRICAL LIGHTING PLAN - AREA A	5/19/2023	
2.12	ELECTRICAL LIGHTING PLAN - AREA B	7/21/2023	
2.13	ELECTRICAL LIGHTING PLAN - AREA C	5/19/2023	
2.13	ELECTRICAL LIGHTING PLAN - AREA D		
		5/19/2023	
2.21	ELECTRICAL POWER PLAN - AREA A	5/19/2023	
2.22	ELECTRICAL POWER PLAN - AREA B	7/21/2023	
2.23	ELECTRICAL POWER PLAN - AREA C	5/19/2023	
2.24	ELECTRICAL POWER PLAN - AREA D	6/30/2023	
2.31	ELECTRICAL EQUIPMENT PLAN - AREA A	5/19/2023	
2.32	ELECTRICAL EQUIPMENT PLAN - AREA B	7/21/2023	
2.33	ELECTRICAL EQUIPMENT PLAN - AREA C	5/19/2023	
2.34	ELECTRICAL EQUIPMENT PLAN - AREA D	5/19/2023	
2.40	ELECTRICAL ROOF PLAN	5/19/2023	
5.00	ELECTRICAL DETAILS	5/19/2023	
5.01	ELECTRICAL DETAILS	5/19/2023	
6.00	ELECTRICAL SCHEDULES	7/21/2023	
6.01	ELECTRICAL SCHEDULES	7/21/2023	
6.10	ELECTRICAL LIGHTING SCHEDULES	5/19/2023	
6.11	ELECTRICAL LIGHTING SCHEDULES	7/21/2023	
	ELECTRICAL ONE-LINE DIAGRAM	7/21/2023	

ISSUED REVISED

5/19/2023

KEYNOTES / GENERAL NOTES

General Notes

PRIOR TO BEGINNING WORK, CONTRACTOR SHALL PROVIDE A CONSTRUCTION SCHEDULE SHOWING THE CHRONOLOGICAL PHASES OF HIS WORK AND ALL RELATED WORK FOR T COMPLETION OF THE PROJECT. THIS SCHEDULE SHA INDICATE ALL ORDERING LEAD TIME, LENGTH OF TIME IN EACH PHASE, IT'S START AND COMPLETION WITH A PROJECTE COMPLETION DATE.

ALL PLANS ARE DRAWN TO SCALE AS MUCH AS POSSIBLE BU ARE NOT INTENDED TO BE AND SHOULD NOT BE SCALED CONTACT ARCHITECT IF ANY DISCREPANCIES WIT DIMENSIONS OR NOTES PRIOR TO COMMENCING COMPLETING WORK.

ALL CONTRACTORS TO CLEAN ALL AREAS DURING AND AFTER CONSTRUCTION TO MAKE READY FOR OTHER TRADE FOLLOWING. AT FINAL PHASE, LEAVE AREA CLEAN FOR MOVE

THE CONTRACTOR IS RESPONSIBLE FOR THE MEANS METHODS AND TECHNIQUES OF CONSTRUCTION, SAFE PRECAUTIONS IN CONNECTION WITH THE WORK AND FOR TH ADDS AND OMISSIONS OF THE SUB-CONTRACTORS.

CONTRACTOR TO FIELD VERIFY ALL DIMENSIONS PRIOR TO INSTALLATION OF ANY ITEM. SUB-CONTRACTOR SHALL REVIEW SITE CONDITIONS PRIOR TO BEGINNING ANY PORTION (

ALL PORTIONS OF THE WORK REQUIRING MAINTENANCE OR

SPARE PARTS REQUIREMENTS SHALL BE SUBMITTED BY TH CONTRACTOR TO THE OWNER. ALL MISCELLANEOUS METAL SHALL BE GALVANIZED (HOT DIPPED) IF EXPOSED TO EXTERIOR OF BUILDING OR PRIME

ALL NOTES AND DETAILS MARKED "TYPICAL" APPLY TO SIMILA CONDITIONS THROUGHOUT THE PROJECT WHETHE SPECIFICALLY NOTED OR NOT. CONTRACTOR SHALL REQUES CLARIFICATION FROM THE DESIGN TEAM IF NEEDED

WITH TWO (2) COATS OF APPROVED PAINT IF EXPOSED

PUBLIC VIEW, OR PRIME COATED IN AREAS NOT TO PUBLIC

PROPERLY ENSURE THE CORRECT DETAIL OR SYSTE ROOF REGULATES AND ROOF FLASHING SHALL BE GALVANIZED STEEL AND INSTALLATION COORDINATED WIT ROOFING INSTALLER.

CONTRACTOR SHALL ENCLOSE AND SEAL ALL MISCELLANEOU OPENINGS AROUND WALL OR FLOOR PENETRATIONS FO PIPING, ELECTRICAL OR MECHANICAL, INTERIOR OR EXTERIO

DIMENSIONS ON FLOOR PLANS ARE FACE TO FACE OF STUDS UNLESS SHOWN OTHERWISE

WITH THE PROPER APPROVED METHOD

PROVIDE BLOCKING BEHIND ALL TOILET ACCESSORIES MIRRORS, WALL CABINETS, AND ANY OTHER ITEM WHICH WIL REQUIRE SECURE ATTACHMENT.

TRANSITION IN DIFFERENT FLOOR MATERIALS SHALL OCCUR

UNDER DOORS EXCEPT WHERE NOTED OTHERWISE

PROVIDE CASING BEADS ON GYPSUM BOARD EDGES WHERE WALLS ABUT MASONRY OR CONCRETE, CAULK JOINTS.

COORDINATE WITH ARCHITECT THE LOCATION AND THE INSTALLATION OF THE FOLLOWING EQUIPMENT: (SHO DRAWINGS REQUIRED)

FIRE ALARM SYSTEM (INCLUDING PANELS, STATIONS,

SMOKE DETECTORS).

EMERGENCY CALL SYSTEM (INCLUDING PANELS. PHONE OUTLETS) TELEPHONE SYSTEM (INCLUDING PANELS & PHONE

SPECIAL SYSTEMS (INCLUDING STRUCTURED PANELS

HOSE BIBBS TO BE MOUNTED AS INDICATED ON FLOOR PLA

SHEET AT AN ELEVATION APPROX. 1'-6" ABOVE FINISH GRADE.

INSULATE ALL PIPING EXPOSED TO OUTSIDE AIR.

PROVIDE BLEED VALVE IN PLUMBING; COORDINATE LOCATION

FIREBLOCKING SHALL BE APPLIED AT OPENINGS AROUND VENTS, PIPES AND DUCTS AT CEILING WITH BATT INSULATION

TO RESIST THE PASSAGE OF FLAME. . INSULATION SHALL HAVE A FLAME SPREAD INDEX OF NOT

MORE THAN 25 AND A SMOKE DEVELOPMENT INDEX OF NOT MORE THAN 450

Drawing Organization G = GENERAL

A0 = SITE PLAN A1 = P-TYPES / SCHEDULES A2 = PLANS

& DATA OUTLETS)

A3 = EXTERIOR ELEVATIONS A4 = SECTIONS A5 = INTERIOR ELEVATIONS A6 = INTERIOR DETAILS

A7 = 3D VIEWS

100% CONSTRUCTION SET

SEAUX+PIERCE architecture

AUSTIN, TX. 78734

■ Burnet City Hall

301 East Jackson Street Burnet, Texas 78611

DATE | REVISED | NOTES CONSTRUCTION SET - PERMIT

REVISION 1 - CITY COMMENTS BID SET W/ VE REVISIONS 4 11/27/23 ADDENDUM

2023 · SEAUX+PIERCE architecture



PROJECT NO. 2021-16 PROJECT INFORMATION

WWW.SEAUXPIERCE.CO

28 Nov 2023 | 18:32:43

GENERAL

- Dimensions refer to rough surfaces. The contractor must verify all dimensions and elevations prior to start of construction. The engineer shall be notified of any discrepancies or inconsistencies.
- All drawings are considered part of the contract documents. The contractor shall be responsible for review and coordination of all drawings and specifications prior to start of construction. Any discrepancies that occur shall be brought to the attention of the engineer prior to the start of construction so that clarifications can be issued. Any work in conflict with contract documents or any code requirements shall be corrected by the con-
- tractor at his own expense and at no expense to the owner or structural engineer. All work shall conform to the minimum standards of the building code as well as any other regulating authority over any portion of the work including those additional codes and standards listed in the structural notes and
- The engineer shall not control and shall not be responsible for construction means, methods, techniques, sequences, or procedures; for safety precautions and programs in connection with the work, for the acts or omissions of the contractor, subcontractor, or for any persons performing the work, or for the failure of any of them to carry out the work in accordance with the contract documents.
- Site observations by field representatives of the engineer are solely for the purpose of determining if the work of the contractor is proceeding in accordance with the structural contract drawings. This limited site observation should not be constructed as exhaustive or continuous to check the quality of the work, but rather an effort to guard the owner against defects or deficiencies in the work of the contractor.
- All structures require periodic maintenance to extend life span and to ensure structural integrity from exposure to the environment. A planned program of maintenance shall be established by the building owner. This program shall include items such as painting of structural steel, protective coating for concrete, sealants,
- caulked joints, expansion joints, control joints, spalls, and cracks in concrete. Refer to Architectural, Mechanical, Electrical and Plumbing drawings for additional information not shown in the structural drawings. Notify engineer of any discrepancies.
- Contractor shall coordinate the requirements for building equipment supported on or from the structure. Submittals identify all equipment supported on or from the structure. Submittals identify all equipment including size, dimensions, clearances, accessibility, weights, and reactions. Any deviations from specified equipment shall be noted on the submittals.
- Shop drawings shall be prepared for all structural items and submitted for review by the Engineer. Contract Drawings shall not be reproduced and used as shop drawings. All items deviating from the Contract Drawings or from previously submitted shop drawings shall be noted.
- The details designated as "Typical Details" apply generally to the Drawings in all areas where conditions are similar to those described in the details.
- The design and provision of all temporary supports required for the execution of the contract such as guys, braces, shores, reshores, falsework, supports and anchors are not included in these drawings and shall be the responsibility of the Contractor. Temporary supports shall not result in the overstress or damage to the struc-

REQUIRED SUBMITTALS

- CONCRETE REBAR SHOP DRAWINGS
- CAST-IN-PLACE ANCHOR RODS STEEL STRUCTURAL EMBEDS
- STEEL STRUCTURAL SHOP DRAWINGS

REQUIRED OBSERVATIONS BY ENGINEER OF RECORD

The structural engineer of record, or his designate, shall provide structural observation of the structural system for general conformance to the approved plans and specifications at significant construction stages:

- PREPOUR OF FOUNDATION
- STEEL FRAMING COLD-FORMED STEEL FRAMING, PRIOR TO BUILDING WRAP

The structural observation is an integral component of the oversight of the construction of the project. If the observations are not performed due to negligence of the owner or contractor, or the contractor does not address the issues raised by the engineer of record at the structural observation, the engineer of record is released of any claims regarding the structural design.

SUBSTITUTIONS

All requests for substitutions of materials or details shown in the contract documents shall be submitted for approval during the bidding period. Once bids are accepted, proposed substitutions will be considered only when they are officially submitted with an identified savings to be deducted from the contract.

BEYOND SCOPE OF STRUCTURAL ENGINEER

The following items are beyond the scope of the structural engineer and are therefore the responsibility of others. The client is responsible for arranging for the design of these systems. Any mention of these items on these drawings is for information purposes only and does not relieve the client of these responsibilities.

- Drainage systems including surface drainage, any area inlets, grate drains, french drains, and subgrade drain-
- Waterproofing systems including vapor barriers, roofing, flashing, waterproofing, and drip edges.
- Ventilation of crawlspace and attic Glazing design and attachment

CODES

All work shall be performed in accordance with applicable sections of the 2015 edition of the International Building Code (IBC 2015), all local amendments to the Code per City of Burnet, and all referenced codes, specifications, and

- Structural Concrete: ACI 318-14 "Building Code Requirements for Reinforced Concrete"; American Concrete
- Structural Steel: ANSI/AISC 360-10" Specification for Structural Steel Buildings", as published in the Manual of teel Construction 15th Edition; American Institute of Steel Construction.
- Structural Cold Formed Steel: AISI S100-12 "North American Specification for the Design of Cold Formed Steel Structural Members, 2012"; American Iron and Steel Institute.

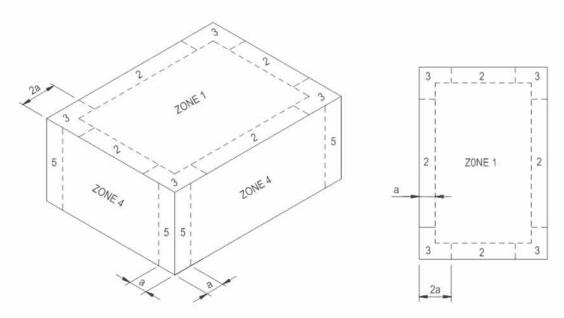
LOADS

Risk Category of Building per 2015 IBC Table 1604.5 = II Wind Loads - Main Wind Force Resisting System:

ISOMETRIC VIEW

Wind Load Design Variables Basic Wind Speed (3 second gust, mph) Exposure Category Internal Pressure Coefficient, Cpi +/- 0.18 Topographic Factor, Kzt

Wind Loads – Components & Cladding:



COMPONENT AND CLADDING WIND LOAD PER AREA (SQ. FT)

PLAN VIEW

EFFECTIVE WIND AREA (SQ. FT)					
a = X'-XX"	10	20	50	100	500
Zone 1	24	24	23	22	22
Zone 2	40	36	30	26	26
Zone 3	60	59	36	26	26
Zone 4	24	23	22	20	19
Zone 5	29	27	25	23	19

Components and Cladding Wind Pressures are based on ASCE 7-16 Chapter 30 Part 3: Buildings with h > 60 ft.

- Components and Cladding zone locations are based on ASCE 7-16 Table 30.7-2 for Flat Roofs
- For parapets around the perimeter of the roof equal to or higher than 3 ft, Zone 3 shall be
- treated as Zone 2. All Parapet Components and Cladding Wind Pressures shall be determined through ASCE 7-16
- Earthquake Loads Seismic design lateral Loads on structural frames are based on the following:

Seismic Importance Factor	1.0
Mapped Spectral Response Accelerations	
S/S	0.058g
S/1	0.033g
Site Class	D
Spectral Response Coefficients	
S/DS	0.062
S/D1	0.053
Seismic Design Category	ΑΑ

Live Loads – Commercial

Location/Element	Live Load (psf)	Remarks & Footnotes (e)
Handrails & Pedestrian Guardrails	50 plf or 200 lb	(a)
Stairs & Exits	100 psf or 300 lb	Stair treads per note (b)
Vehicle Barrier	6000 lbs	Applied horizontally at both 18" and 27" above the level (c)
Lobbies	100	2000 lbs
Corridors at First Floor	100	
Offices	50 + 15	2000 lbs (d)
Fixed-seat Courtroom	60	
Mechanical Rooms	150	
Light Storage Area	125	
Roofs	20 psf or 300 lb	Area load is reducible. Point load per note (b), See below for Snow Load

- Top rail shall be designed to resist 50 PLF line load or 200 lb point load applied in any direction at any point. Intermediate rails (all those except the handrail), balusters and panel fillers shall be designed to withstand a horizontally applied normal load of 50 LB on an area not to exceed 1 ft square. These three loads are to be considered separately with worst case used
- Place 300 lb concentrated load over 2"x2" area at any point to produce maximum stress. Area load and concentrated load are to be considered separately with worst case used for design.
- Need not apply concurrently with other handrail and guardrail loads; applied over not more than 1 square foot.
- Floors for Business Group B (Offices) Occupancy shall be designed with a basic floor Live Load plus an additional 15 PSF (minimum) live loading for moveable partitions. Unless otherwise noted, point loads to be distributed over a 2.5ft x 2.5ft area and located to produce maximum load effects on structural members.

Dead Loads:

Location/Element	Dead Load (psf)	Remarks
Roofing (TPO)	8	
Roof Total Top Chord Bottom Chord	25 psf 15 psf 10 psf	For open web steel joist design, 500 lb (a).
Brick Veneer	50	

Point load to be applied at any panel point on the top or bottom chord of each open web steel joist (wherever it produces the highest stress).

Snow Loads:

Snow Load Design Variables	Value	
Flat Roof Snow Load, psf	20	
Ground Snow Load, psf	5	
Snow Importance Factor	1.0	

BUILDING PAD

- Building pad preparation information is based on a geotechnical report provided by Holt Engineering, Inc., File No. 01-00823 dated January 26, 2023. Prior to excavating for building pads or placing any fill soils, all organic materials, existing pavements, and otherwise unsuitable materials shall be removed from planned building areas to a depth of 36" minimum below
- final grade and 12" minimum below existing grade. Site stripping shall include the limits of any proposed uilding and abutting sidewalks or flatwork, plus a horizontal distance of 3 feet beyond. Concrete slab to be built over 36" of select compacted fill.

 Bottom of exterior grade beams shall be founded 48" BELOW GRADE. Bottom of grade beams shall have a slope less than or equal to 1 in 10. Under no circumstances shall concrete
- beams be placed on sloping grade greater than 1 in 10. Bottom of grade beams shall be free of loose deleterious fill material including topsoil, loose rocks, crushed rock, base material, water, or moist soil. Place imported select fill in approximately 8-inch loose lifts, watered as required and compacted to 95 percent of maximum dry density (as defined in ASTM D 698) at a moisture content within -3 to +3 percent of optimum
- moisture content. Compacted thickness of each lift should not exceed 6 inches. Grade adjustments within the building limits shall be accomplished with select fill soils meeting TxDOT standard specifications Item 247, Type A, Grade 4 (Crushed Limestone Base Material). All structural fill shall be placed on prepared surfaces in lifts not to exceed eight inches loose measure with compacted thickness not to exceed six inches. The fill shall be compacted to at least 95 percent of the ASTM 698 maximum dry density at a moisture content ranging between -2 and +3 percent of optimum moisture content.
- Where not covered by concrete flatwork or pavements, provide 2-foot-thick clay caps at overbuild areas along the perimeters of slabs-on-grade over building pads, to protect from moisture intrusion. Caps shall slope away from buildings.
- Provide a 10-mil vapor barrier placed according to manufacturer's recommendations between the bottom of slab and the top of the select fill. Moisture barrier shall not be draped continuous across the bottom of grade
- 10. Foundation slab concrete should be placed within 2 weeks of the completion of trench excavations and the moisture barrier should be installed before any notable rainfall event. If the bearing soils are softened by surface water intrusion or disturbance, the softened soils must be removed from the foundation excavation bottom prior to concrete placement. Exposure to the environment may weaken the soils at the grade beam bearing level if the foundation excavations remain open for an extended duration.

DRILLED CONCRETE PIERS

- Drilled concrete piers are designed based on a geotechnical report provided by Holt Engineering, Inc, File No: 01-00323 dated January 26, 2023. 5,000 psf (below existing grade) Allowable net end bearing
- All drilled concrete piers shall bear 10'-0" BELOW EXISTING GRADE. Each pier shaft shall be inspected by a representative of FORT STRUCTURES to insure clean intact bearing.
- All piers shall be centered on beams unless noted otherwise. Contractor shall drill piers to the exact size shown. Shafts shall be drilled plumb with a tolerance of two inches. Footing and pier bottoms shall be thoroughly clean and free of water when concrete is placed. For estimating purposes, carry all piers 10'-0" feet below existing grade when directed by the Engineer, carry piers to greater depths to provide suitable bearing. The engineer reserves the right to increase the depth of piers if bearing surface is deemed unsuitable. Adjustments will be made in the contract price for more depth
- in accordance with the unit prices quoted in the contractor's bid. Contractor shall provide suitable access, lighting, and measuring devices for inspection of the excavations for cleanliness and correctness of dimensions and alignment.
- Provide dowels from piers into concrete above using the same bar size and number as shown for the pier. Pier reinforcing and concrete shall be placed immediately after drilling operations are complete. In no case shall a pier be drilled that cannot be poured by the end of the workday. Temporary steel casing may be required during pier drilling operations if water is encountered. Follow spe-
- cial construction procedures in accordance with ACI 335.1-98. The contractor shall make and maintain accurate records of the drilled pier depths, bearing stratum, depth of penetration into bearing stratum, diameter, and location, and shall submit this information to the Engineer.

CONCRETE FOOTINGS

- Foundations have been designed based on a geotechnical report provided by Holt Engineering, Inc, File No:
- 01-00323 dated January 26, 2023. Concrete footing design is based on the following allowable net bearing capacities: Bearing 48" minimum below existing grade
- Bearing 60" minimum below existing grade 3,000 psf Bearing stratum shown on the footing details is 48" minimum embedment into existing grade. Footings not specifically located on the plan shall be located on centerline of pilaster or column above.
- Where no pilaster or column occurs, locate on centerline of wall or beam. Elevation of top of footings, unless noted otherwise on drawings, is at the bottom of the deepest intersecting
- beam or wall supported by the footing
- Footing excavations shall be to neat lines and shall be free of loose or wet materials Concrete should be placed within 2 weeks of the completion of footing excavations and the moisture barrier
- should be installed before any notable rainfall event. If the bearing soils are softened by surface water intrusion or disturbance, the softened soils must be removed from the foundation excavation bottom prior to concrete placement. Exposure to the environment may weaken the soils at the grade beam bearing level if the
- See plans and schedules for footing sizes, reinforcing and depths. All footings shall be inspected by a representative of Fort Structures in order to ensure that the proposed bearing material has been reached in accordance with the plans and that the footing has been constructed to specified size, with detailed reinforcing, and to specified tolerances.

CAST IN PLACE CONCRETE

- Comply with the provisions of the following latest codes, specifications, and standards, except as otherwise
- ACI 301 "Specifications for Structural Concrete for Buildings"

oundation excavations remain open for an extended duration.

- ACI 311 "Recommended Practice for Concrete Inspection" ACI 318 "Building Code Requirements for Reinforced Concrete".
- ACI 347 "Recommended Practice for Concrete Formwork". ACI 304 "Recommended Practice for Measuring, Mixing, Transporting and Placing Concrete".

Use/Location	Strength f'c (psi)	Testing Age (days)	Max Aggre- gate Size	Exposure Class	Type	Slump
Piers	3000	28	1"	-	C33	3"-5"
Slabs on Grade	3000	28	1"	Α	C33	3"-5"
Site Retaining Walls	3000	28	1"	Α	C33	3"-5"

- Provide 3 percent plus or minus 1½ percent of entrained air in concrete permanently exposed to the weather. Contractor shall develop and submit a hot weather concreting plan for approval. Follow ACI 305 recommendations in developing hot weather concreting plan.
- Proper consolidation shall be achieved through externally vibrating the forms, vibrating the wet concrete or by other appropriate means.
- Embedded conduits, pipes, and sleeves shall meet the requirements of ACI 318-19, Section 6.3, including the Conduits and pipes embedded within a slab, wall, or beam (other than those passing through) shall not be larger in outside dimension than 1/3 the overall thickness of the slab, wall, or beam in which they are
- Conduits, pipes, and sleeves shall not be spaced closer than three diameters on center. Concrete pours shall not exceed 8000 square feet or 100 linear feet on each side without prior approval by
- the Architect for each pour or noted on plan. Submittal: Submit proposed mix designs in accordance with ACI 301, chapter 3.9. Each proposed mix design shall be accompanied by a record of past performance based on at least 30 consecutive strength tests, or by three laboratory trial mixtures with confirmation tests.
- Contractor shall coordinate all exposed concrete with architectural finish and specifications. Contractor shall submit concrete curing procedure for all architecturally exposed concrete.
- The contractor is responsible for correction of concrete work which does not conform to the specified requirements, including strength, tolerances, and finishes. Correct deficient concrete as directed by the architect.

CONCRETE REINFORCEMENT

- Reinforcing steel shall be deformed new billet steel bars in accordance with ASTM A615 Grade 60. Detailing of reinforcing steel shall conform to the American Concrete Institute Detailing Manual. All hooks and bends in reinforcing bars shall conform to ACI detailing standards unless shown otherwise. Provide reinforcing bars in accordance with the bar bending diagram if bar types are specified. In unsched-
- uled beams, slabs, columns, and walls detail reinforcing as follows Lap top reinforcing bars at mid span. Lap bottom reinforcing bars at the supports.
- Lap vertical bars in columns and walls only at floor lines, unless noted otherwise. Refer to lap splice schedule for splice length requirement. Reinforcement labeled as continuous shall be lap spliced 38 bar diameters as a minimum, unless other-
- Provide standard hooks in top bars at cantilever and discontinuous ends of beams, walls, and slabs, Provide corner bars for all horizontal bars at the inside and outside faces of intersecting beams or walls.
- Corner bars are not required if top, bottom, or horizontal bars are hooked. Welding of reinforcing steel will not be permitted Heat shall not be used in the fabrication or installation of reinforcement.
- Reinforcing steel clear cover shall be as follows: Concrete cast against earth Concrete exposed to earth or weather Ties in columns and beams
- Submittal: Submit shop drawings for fabrication, bending, and placement of concrete reinforcement. Comply with ACI 315 "Details and Detailing of Concrete Reinforcement". Do not reproduce the Contract Drawings for use as shop drawings.

STRUCTURAL STEEL

Bars in slabs

Bars in walls

- Structural Steel shall conform to ASTM A992 or A572, grade 50 except where A36 is noted on plan, except that miscellaneous plates, angles, and channels may be A572, grade 50 or A36. Steel pipe shall conform to ASTM Specification A 501 or ASTM A 53, Type E or S, Grade B. Steel tube shall conform to ASTM Specifica-
- tion A 500, Grade B, Fy 46 ksi. Anchor bolts shall conform to ASTM F1554 grade 36 ksi. Column base plates shall be grouted with a non-shrink, high strength nonmetallic grout conforming to ASTM
- C827, and shall have a compressive strength at 28 days of 5000 psi. Pre-grouting of base plates will not be Studs shall be Nelson stud type S3L (Fu=65 ksi) or acceptable equal. Studs shall be made from cold drawn
- steel conforming to ASTM A108. Deformed bar anchors shall be Nelson D2L or KSM deformed bar anchors (or acceptable equal) and shall be made from cold drawn wire per STM A490 conforming to ASTM A108 with minimum yield strength of 70 Ksi. Anchors shall be automatically and welded with suitable welding equipment in the shop or in the field. Weld-
- ing shall be in accordance with the recommendations of Nelson Stud Company or KSM Welding Company. Structural steel detailing, fabrication, and erection shall conform to the AISC "Specification for Steel Buildings" and the AISC "Code of Standard Practice for Steel Buildings and Bridges" except that paragraph 4.2.1 "the owner's acceptance of all responsibility for the design adequacy of any connections designed by the fabricator" is deleted. Typical connection details are indicated in the drawings. The fabricator shall prepare drawings based on these details. If alternate connection designs are used, the fabricator shall have a regis-
- tered professional engineer prepare the connection designs. Such connection shall bear the engineer's seal and shall be submitted with shop drawings. Splicing of structural steel members is prohibited without prior approval of the Engineer as to location and type of splice to be made. Any member having splice not shown and detailed on shop drawings will be reject-
- All welds denoted as moment connection or full penetration weld shall be ultrasonically or x-ray certified by an
- independent testing agency. Contractor shall coordinate structural steel fireproofing requirements. All interior structural steel, including steel joists, scheduled, or indicated to receive spray applied fireproofing shall be delivered to the project site unprimed. Steel exposed to corrosive conditions after installation shall be primed with a protective coating which does not diminish the bond between the spray applied fireproofing, and the steel substrate. Any primer, and/or coating applied to structural steel shall be approved for use in the applicable U.L. Fire Resistance Assembly used on the project. Contractor shall protect any unprimed structural steel from detrimental effects
- of corrosion, as required, until the steel is enclosed and protected by the new construction. Shop painting: Paint structural steel with one coat of manufacturer's standard red oxide primer applied at a rate to provide a uniform dry film thickness of 2.5 mils. Ref. Arch for Finish Coat Submittal: Provide drawings showing details for fabrication and shop assembly of members, erection plans and details. Include details of connections, camber, weld profiles and sizes and spacing. Shop and erection
- drawings shall not be made using reproductions of the contract drawings Contractor must fabricate and erect steel in accordance with OSHA Safety requirements, 29 CF part 1926 Safety for Steel Erection, Final Rule.

STRUCTURAL STEEL CONNECTIONS

- Welding shall conform to ANSI/AWS D1.1, latest edition. Bolts conform to ASTM A325. Bolts shall be designed using values for bearing type bolts with thread allowed
- Structural steel connections not specifically detailed on the Drawings shall be designed and detailed by the Contractor under the direct supervision of a registered engineer licensed in the State of Texas. Sealed calculations for all connections designed by the Contractor shall be submitted for the Architect's files. Connections that meet the requirements and assumptions presented in our schematic connection details and table can be used at the discretion of the Contractor. The Contractor shall take full responsibility in confirming that the con-
- nection tables are used within their limitations and assumptions outlined in the details and notes. Beam connections shall be designed and detailed as follows, unless noted otherwise on the Drawings:
- Where indicated, connections shall be designed for the scheduled shear force, the shear force indicat-
- ed on the Drawings as "V= ", and the horizontal force indicated as "H= '
- for the beam span shown in the beam tables in Section 2 of the AISC Manual, ninth edition. The minimum number of rows of bolts shall be 1/6 of the beam depth with any fraction be rounded to
- ments. Washers shall be hardened where A325 bolts are utilized.
- Connections shall be welded. Connections shall be designed and detailed for the forces shown on the Drawings.
- capacity of the members.
- surfaces sufficient to develop the tensile strength of the smaller member at the joint.
- ber on both sides of supporting member.
- develop full capacity of the member.

OPEN WEB STEEL JOISTS AND JOISTS GIRDERS (OWSJ)

- IBC Section 2207 "Steel Joists"
- SJI "Standard Specifications for Open Web Steel Joists, K-Series" SJI – "Standard Specifications for Long span Steel Joists, LH-Series and Deep Long span Steel Joists,
- SJI "Standard Specifications for Joist Girders"
- 'STRUCTURAL GENERAL NOTES". Design shall include the effects of wind up-lift as well as drifting and sliding snow, when applicable, in accordance with IBC Section 1608, as shown on the structural plans, and for fire
- sprinkler support loads, where applicable. Deflection limits shall be as per IBC Section 2207. The overall stability of the joist system is the responsibility of the OWSJ supplier. Careful attention shall be given to the stability of the joists during erection in accordance with the IBC and all sections of the SJI Manual. Specifically, sections in the SJI Manual on "Bridging", "Erection Stability and Handling" and "Handling and rection" shall be carefully followed by the SSE (SJI supplier) to provide stability of all members at all times.
- All bridging, collector-drag struts, drag splice plates, bottom chord bracing, girders and related connection hardware shall be provided and designed by the supplier. All additional erection bolts, stabilizer plates, and any other additional steel to meet OSHA standards, shall be coordinated by the joist manufacturer and shall be provided by the steel detailer/supplier. Supplier to provide sloped bearing seats where required for roof slope. Reference drawings for non-standard joist end bearing lengths.

METAL ROOF DECKS

- Metal deck shall be galvanized per ASTM A525, class G60coating. Metal deck shall by "Vulcraft" type as shown on plan or approved equal. Deck shall be continuous over 4 or more supports so as not to require any intermediate shoring to support
- sired to eliminate shoring requirements.
- Metal edge forms shall be as indicated on plan and details, minimum 14-gauge cold formed steel with 1/2" return lip. Weld edge form to supporting beams at 12" spacing, unless noted otherwise.
- Deck manufacturer shall furnish shoring plans, closure plates, ridge and valley plates, cant strips, sump pans, flashing and all other light gage steel material required to complete the work.
- 1/2" diameter puddle welds or #12 tek screws each rib at transverse and perimeter supports. 1/2" diameter puddle welds or #12 teks screws at 6" O.C. at longitudinal supports,
- Side lap connections necessary to develop the shear loading indicated on the diaphragm schedule, but
- not less than 600 PLF.
- Studs and track shall be galvanized in accordance with ASTM A653, G60, unless in contact with pressure treated wood. If in contact with pressure treated wood, use G90 or greater coatings.
- flection track to allow for a differential vertical load deflection under live load conditions. A 3/4" gap between
- top of studs and slab is required unless noted otherwise on plan. Connectors shall be in conformance with the manufacturer's specifications. Screws shall be snug with the steel surface and shall penetrate into steel studs by a minimum of three exposed threads. Screws shall be in-
- When fastening to steel, Powder Actuated Fasteners shall be installed a minimum of 1/2" from steel edges and with no less than 1" O.C. spacing.
- Powder Actuated Fasteners shall not be used for hanging applications. Cold-formed steel framing shall be erected true and plumb per the requirements and within the specified tolerances listed below. For purposes of this section, camber is defined as the deviation from straightness of a
- a. Joists, track, and axial load bearing studs, out of plumbness and out of straightness (camber and sweep) shall not exceed 1/1000th of the member length (1/8" over 10'-0") Studs shall seat into top and bottom tracks. The gap between the end of the stud and the web of the
- track shall not exceed 1/16" for axial load bearing studs. Joists and end stiffeners shall be located directly over studs. The use of a wall top track as a load distri-

bearing points. If a punch-out falls within 10" of a bearing point, reinforcement is required.

- MASONRY VENEER
- Maximum wall height: Install over backing of wood wall and limited in height to a maximum of 30 feet above foundation (38 feet permitted at the top of gable ends). Masonry on Wood: Where indicated on plans, masonry weighing less than 40 psf may be supported on wood
- neer supported by the foundation. Anchorage: Anchor to wood wall framing with corrosion resistant 22-gauge X 7/8" corrugated sheet metal
- ties spaced a maximum of 24" horizontally and 19.5" vertically.

Addendum 2 11.27.2023 NO ISSUE DATE

FIRM REGISTRATION NO. 18034

WWW.FORTSTRUCTURES.COM

SHANE L. TANNER

ARCHITECT

PROJECT NO: 22054

SEAUX+PIERCE

300 East Jackson Street

PERMIT SET

05.19.2023

Burnet, Texas 78611

QA/QC: S. Tanner If printed on 22x34 or 24x36 sheet,

STRUCTURAL GENERAL

NOTES

SHEET TITLE

Connections shall be AISC type 2 simple framing connections. In general, shop connections shall be bolted or welded, and field connections shall be bolted.

If not indicated on the Drawings, connections shall be designed for 55 percent of the total load capacity

the next higher number. Short slotted holes shall be permitted provided washers are installed in accordance with AISC require-

Wind brace and truss connections shall be designed and detailed as follows, unless noted otherwise on the

Moment connections indicated on Drawings as "MC" shall be welded to develop the full capacity of the mem-

1014 Sailmaster St. If forces are not indicated on the Drawings, connections shall be designed to develop the full tensile Austin, Texas 78734 For connections not specifically addressed by these notes or the Drawings, provide fillet welds at all contact www.seauxpierce.com

Roof edges angles shall be continuous and shall be spliced only at supports. Splices shall be butt-welded to

Fillet welds with no size specified shall be 3/16", or minimum size required by AISC, whichever is larger. **BURNET CITY HALL**

All OWSJ shall conform to the following specifications:

COPYRIGHT © 2023 FORT STRUCTURES

Provide shop drawings and submittals complying with IBC 2207 with structural calculations stamped by a Professional Engineer registered in the Authority Having Jurisdiction. Joists and girders shall be designed and fabricated by a member of the Steel Joist Institute (SJI) for the loads indicated above and on the drawings. Design gravity loads are listed under the "LOADS" section in these

- Metal deck shall be as specified on plan. Minimum section modulus shall be 0.34 in³ per linear foot of deck.
- construction loads and wet concrete, unless noted otherwise. Deck continuous over fewer than four supports shall be shored from the supporting beams. Two span deck shall not be used. Contractor may submit properly designed heavier gauge deck installed in single span lengths for approval from structural engineer, if de-
- Deck openings less than 6" do not require reinforcement. For larger openings, refer to typical details.
- Minimum deck fastening shall be as follows, unless noted otherwise on drawings:
- COLD-FORMED STEEL FRAMING Cold-formed steel framing shall be as specified on the plans and details and in accordance with the Steel Stud Manufacturer's Association ICC Evaluation Report ER-4943P.
- Damaged members, members with cracking in the steel at the bend radius locations, and members with significant red rusting or scaling of protective coating are unacceptable and must be replaced, unless approved
- Field cuts and notches of any kind are NOT allowed in any structural cold-formed steel member without prior Non-load-bearing stud walls shall be attached to the concrete slab, metal deck or steel beams above with de-
- stalled a minimum of 3/8" from steel edges and at a spacing of no less than 3/4" O.C.
- When fastening to concrete, Powder Actuated Fasteners shall be installed a minimum of 3" from concrete edges and with no less than 4" O.C. spacing.
- member of any portion of a member with respect to its major axis, and sweep is defined as the deviation from straightness of a member or any portion of a member with respect to its minor axis.
- bution member is not permitted. Provide C-shaped joists with stiffened flanges (S-sections in SSMA). Spans are assumed to be continuously sheathed with metal deck at the top flange. All joists must be braced laterally at each end by track or blocking. Joist bridging shall be a maximum of 8'-0" O.C. Web punch-outs shall be located a minimum of 10" away from

- framing (deflection limit L/600). Install a movement joint between the veneer supported by wood and the ve-
- Lintels over Openings: Support Masonry on loose lintels per code supported on masonry to foundation at

Isolation Joints: Install vertical isolation/expansion joints at approximately 25 feet on center.

PM: S. Tanner ENG: P. El Hanna BIM PM: C. Hernandez

the scale is as indicated. If printed on an 11x17 or 12x18 sheet, the scale is reduced by half. SCALE

SHEET NUMBER

ADHESIVE ANCHORS

- Adhesive anchors shall only be used where specified on the drawings. The Contractor shall obtain approval from the engineer of record prior to using the anchors for missing or misplaced cast-in-place anchors.
- 2. Unless otherwise noted, size and depth of the adhesive anchors specified on the drawings are based on HAS rods and the following epoxy systems: a. CONCRETE EPOXY
 - Hilti HIT RE-500 V3
 - DeWalt PurePro 110+ Simpson Set-3G
- Substitution of expansion anchor products with similar capacities shall be submitted to the engineer of record
- Adhesive anchors of the size and embedment shown on the Drawings shall be installed in accordance with the Contract Documents, the manufacturer's recommendations, and the manufacturer's current ICBO report for the anchor. If conflicts exist between these referenced documents, the most stringent requirements shall
- Contractor shall locate all existing reinforcing steel and other embedded items contained in the concrete using non-destructive methods and shall position anchor locations to avoid conflicts with existing embedded items. Anchor locations can be adjusted by a maximum of 1 inch from detailed locations to avoid conflicts, unless noted otherwise.
- Based on field verified locations of reinforcing steel and embedded items, the Contractor shall create templates for each anchor group. Submit template dimensions for review prior to fabrication of connection plates.
- Holes for anchors shall be drilled in a continuous operation using the bit type and size recommended by the anchor manufacturer. Holes shall be drilled perpendicular to the concrete surface and shall not be enlarged or redirected at any point along its length. All debris shall be blown out of the holes with compressed air after
- All abandoned holes shall be filled with non-shrink grout.

 Holes in connection plates shall be no more than 1/16" larger than the anchor diameter. If larger holes are required for erection purposes, Contractor shall provide 1/4" x 3" x 3" plate washers sufficiently welded to the connection plate to transfer the specified load.
- 10. Installation of adhesive anchors shall be continuously inspected by the testing agency to ensure that holes are of specified size, and that bolts are properly installed.

TESTING LABORATORY SERVICES

- Work specified herein shall be performed by a qualified independent Testing Laboratory, selected and paid by
- Footing excavation: Inspect the excavations to determine that the proper bearing stratum is obtained and utilized for bearing and that excavations are thoroughly clean and dry before concrete is placed.
- Concrete inspection and testing:
- Secure composite samples of concrete at the jobsite in accordance with ASTM C172.
- Mold and cure three specimens from each sample in accordance with ASTM C31. Test specimens in accordance with ASTM C39. Two specimens shall be tested at 28 days for acceptance, and one shall be tested at seven days for information.
- Perform one strength test (three cylinders) for each pour.
- Concrete Reinforcement: Inspect all concrete reinforcing steel and embedded metal assemblies prior to
 placement of concrete for compliance with Contract Documents and shop drawings. All instances of noncompliance shall be immediately brought to the attention of the contractor for correction, and if uncorrected,
- Structural steel, Steel joists and Joist Girders: Field inspection of proper erection of all members, visual examination of all field welding, visual inspection of all bolts, inspection of all shop fabricated members upon arrival at the jobsite for conformance with accepted fabrication and erection drawings, verification of welder's certifi-

CONCRETE STRENGTH TESTING AND ACCEPTANCE

- Obtain samples and conduct tests in accordance with ACI 301 Section 1.6.3.2. Additional samples may be re-
- quired to obtain concrete strengths at alternate intervals than shown below. Perform one strength test for each pour.
- Cure 4 cylinders for 28-day test age: test 1 cylinder at 7 days, test 1 cylinders at 14 days, test 1 cylinders at 28 days, and hold 1 cylinder in reserve for use as the Engineer directs. After 56 days, unless notified by the Engineer to the contrary, the reserve cylinder may be discarded without being tested for specimens meeting 28-day strength requirements.
- The number of cylinders indicated above reference 6 by 12 in cylinders. If 4 by 8 in cylinders are to be used, additional cylinders must be cured for testing of 3 cylinders at test age per the table of mix design requirements.
- Strength is satisfactory when:
- The averages of all sets of 3 consecutive tests equal or exceed the specified strength.
- No individual test falls below the specified strength by more than 500 psi.

 A "test" for acceptance is the average strength of two 6 by 12 in. cylinders or three 4 by 8 in. cylinders
- tested at the specified test age.



WWW.FORTSTRUCTURES.COM PROJECT NO: 22054



SEAUX+PIERCE

1014 Sailmaster St. Austin, Texas 78734 www.seauxpierce.com

ARCHITECT

BURNET CITY HALL

300 East Jackson Street Burnet, Texas 78611

> PERMIT SET 05.19.2023

STRUCTURAL GENERAL NOTES

sheet, the scale is reduced by half. SCALE

11.27.2023

DATE

4 Addendum 2

BIM PM: C. Hernandez QA/QC: S. Tanner

the scale is as indicated. If printed on an 11x17 or 12x18

PM: S. Tanner ENG: P. El Hanna

ISSUE

If printed on 22x34 or 24x36 sheet,

COPYRIGHT © 2023 FORT STRUCTURES

FORT STRUCTURES SHEET SERIES LEGEND				
SHEET SERIES	DESCRIPTION			
S0	STRUCTURAL NOTES			
S1	AXONOMETRIC VIEWS			
S2	PLANS			
S3	ELEVATIONS & BUILDING SECTIONS			
S4	FOUNDATION DETAILS			
S5	ELEVATED CONCRETE DETAILS			
S6	CMU DETAILS/3D PRINTED DETAILS			
S7	STEEL DETAILS			
S8	WOOD DETAILS			
S9	COLD-FORMED STEEL DETAILS			

_						
STR	STRUCTURAL SHEET LIST (22054)					
SHEET						
NUMBER	SHEET NAME					
\$0.01	STRUCTURAL GENERAL NOTES					
S0.02	STRUCTURAL GENERAL NOTES					
2 S0.10	ABBREVIATIONS & LEGENDS					
Ş1.01	AXONOMETRIC VIEWS					
S2.00	PIER LAYOUT PLAN					
S2.01	FOUNDATION PLAN					
S2.02	ROOF FRAMING PLAN					
S2.03	HIGH ROOF FRAMING PLAN & ENLARGED PLANS					
S3.01	BRACED FRAME ELEVATIONS					
S3.02	BRACED FRAME DETAILS					
S4.01	TYPICAL FOUNDATION DETAILS					
S4.02	TYPICAL FOUNDATION DETAILS					
S4.10	FOUNDATION DETAILS					
S4.11	FOUNDATION DETAILS - SITE RETAINING WALLS					
S7.01	TYPICAL BASE PLATE & HSS COLUMN STEEL DETAILS					
S7.02	TYPICAL STEEL BEAM CONNECTION DETAILS					
S7.03	TYPICAL ROOF K OWSJ DETAILS					
S7.10	ROOF FRAMING DETAILS					
S9.01	TYPICAL COLD-FORMED STEEL DETAILS					
S9.02	TYPICAL COLD-FORMED STEEL DETAILS					
Sheet Total: 20						

	FORT STRUCTURES	DRAWING	LEGEND
TAG	DESCRIPTION	SYMBOL	DESCRIPTION
GB1.0	GRADE BEAM (REFER TO GRADE BEAM SCHEDULE)	++	POST-TENSIONED LIVE END
F2.0	FOOTING (REFER TO FOOTING SCHEDULE)		POST-TENSIONED DRAPE
SF2.0	STRIP FOOTING (REFER TO FOOTING SCHEDULE)	—	POST-TENSIONED DEAD END
P24	DRILLED STRAIGHT SHAFT PIER (REFER TO SCHEDULE)		MATCHLINE
P24/UR48	DRILLED STRAIGHT SHAFT PIER WITH UNDERREAM (REFER TO SCHEDULE)		SHORING INDICATION
SDP6	STEEL DRIVEN PILE (REFER TO PLANS & DETAILS)	[A]	ABOVE INDICATION
PC-X	PIER/PILE CAP (REFER TO SCHEDULE)	@	"AT" SYMBOL WHEN INDICATING A SPACING
S	FOOTING/GRADE BEAM STEP	Œ	CENTERLINE INDICATION
BT-XX	POST-TENSIONED GRADE BEAM W/ TENDON LENGTH (REFER TO SCHEDULE)	Ø	DIAMETER INDICATION
ST-XX	POST-TENSIONED SLAB ON GRADE W/ TENDON LENGTH (REFER TO SCHEDULE)	[E]	EXISTING INDICATION
CS-WSP	CONTINUOUSLY SHEATHED-WOOD STRUCTURAL PANEL (REFER TO SCHEDULE)	•	MOMENT CONNECTION
HD-X HD-X	SHEAR WALL POINTS TO NAILING PATTERN SIDE (REFER TO SCHEDULE)	±	PLUS OR MINUS INDICATION
XXXX [E]Type Name	STEEL/CONCRETE COLUMN (REFER TO SCHEDULE)	FLUSH	FLUSH FLOOR INDICATION
BP-X	BASEPLATE (REFER TO DETAIL SCHEDULE)		SLOPE/RAMP DOWN INDICATION
SP-X	SADDLE PLATE (REFER TO DETAIL SCHEDULE)		SLOPE/RAMP UP INDICATION
SR-X	STUD RAIL (REFER TO DETAILS)	X"	STEP DOWN INDICATION
1TB	REINFORCEMENT TYPE (REFER TO SCHEDULE)		ROOF RIDGE INDICATION
MD-X	METAL DECK SPAN DIRECTION (REFER TO SCHEDULE)		ROOF VALLEY INDICATION
RT-X	RIM TRACK (REFER TO SCHEDULE)		
T/XXX = X'-X"	TOP ELEVATION OF ELEMENT		
B/XXX = X'-X"	BOTTOM ELEVATION OF ELEMENT		
XX/SX.XX	SECTION VIEW (DETAIL NUMBER/SHEET NUMBER)		
XX/SX.XX	CALLOUT VIEW (DETAIL NUMBER/SHEET NUMBER)		
XX/SX.XX	ELEVATION VIEW (DETAIL NUMBER/SHEET NUMBER)		
\triangle	REVISION DELTA (REFER TO REVISION SCHEDULE)		

	FORT STRUCTURES MATERIAL PATTERN LEGEND							
+ + + + + + + + + + + + + + + + + + + +	ADDITIONAL LOADING (PLANS)		GRADING (PLANS/DETAILS)		PLYWOOD (DETAILS)			
	CFS BEARING WALL (PLANS)		GRADING UNDISTURBED (PLANS/DETAILS)		POUR STRIP (PLANS)			
	CFS NON-BEARING WALL (PLANS)		GRATING (PLANS)		PRECAST CONCRETE (PLANS/DETAILS)			
	CMU BEARING (PLANS/DETAILS)		GRAVEL (DETAILS)		ROCK (DETAILS)			
	CMU NON-BEARING (PLANS/DETAILS)		GROUT (DETAILS)		STEEL (DETAILS)			
	CAST-IN-PLACE CONCRETE BEARING (PLANS/DETAILS)		3D PRINTED BEARING WALL (PLANS/DETAILS)		WOOD BEARING WALL (PLANS)			
	CAST-IN-PLACE CONCRETE NON-BEARING (PLANS/DETAILS)		3D PRINTED CORE (PLANS)		WOOD NON-BEARING WALL (PLANS)			
	CRITICAL ROOT ZONE FOR NO IMPACTS (PLANS)		MASONRY BEARING (PLANS/DETAILS)					
	EXISTING (PLANS/DETAILS)		MASONRY NON-BEARING (PLANS/DETAILS)					
	FILL (DETAILS)		OVER-FRAMING (PLANS)					

ADDI	AB	_ANCHOR BOLT	GA	_GAGE	R	_RADIUS
ADJ. ADJACENT A FLORENT CONCESSION OF SCHOOL O	ADDL	 _ADDITIONAL	GALV	 _GALVANIZE(D)	RCP	_ _REINFORCED CONCRETE I
ALTERNATE ATTOROMANDATELY ATTOROMANDATELY ATTOROMANDATELY ARCOLLEGE AND		_		_		
APPROVAMENTED PROCESSOR AND ROAD ROAD ROAD ROAD ROAD ROAD ROAD ROA		_				_
ABACTI ARCHITECTURAL) BY DOT ADTOM BY DOT		_				
ABCH_ABCHTECTUREN] BY MOT_BUTTON B				_		_REINFORCE(ING)(ED)(MEN
8 OF 100 MOTOR OF				_		_
8 BOT DOTTOM PE	ARCH	_ARCHITECT(URAL)				-
BOTTOM OF	D DOT	DOTTOM	GYP	_GYPSUM BOARD		_
SEB BOTTOM CHORD BEARING HER		_	LID	LICLD DOWN		_
BACK FACE HG		_		_		
BRACE FRAME		_		_		_
BULDING UNE		_		_	KI	_RIM TRACK
BIDG BILOCKOUT		_		_	SCHED	SCHEDI II E(D)
BILOCKINS		_				
B				_		
BR BASE PLATE HT				_		-
BROOK				-		
BIRD		_		_		SHORT LEG BACK TO BAC
BILL				' '		SPECIAL MOMENT FRAME
BINL BRACK WALL LINE I.E. NISSED DIAMETER SP. SADOLE PLATE SP.		_	IBC	INTERNATIONAL BUILDING CODE		
BANCE WALL LINE				_		
C	BWL	BRACE WALL LINE	IE	INVERT ELEVATION		SPACE(S)(D)(ING)
CANTILEVER INTERM INTERMEDIATE SPINE SOUTHERN PINE COBER COUNTERBORE JONNT SR STUDRAL CO CASTAN-PLACE K KPS STAGGERPIO CL CONSTRUCTION JOINT K KPS STAGGERPIO CL CONSTRUCTION JOINT K KPS STAGGERPIO CL CONSTRUCTION JOINT L ANGLE STAGGERPIO CL CONSTRUCTION JOINT L ANGLE STAGGERPIO CL CLORATION L ANGLE STR STRUCTURER CL CLORATION L LIVIL JONG LEG PROZECTIAL STR STRUCTURER(AL) CONCIL CONGERE LOC LOCATION STR STRUCTURER(AL) CONCIL CONGERE LOC LOCATION STR STRUCTURER(AL) CONCIL CONGERE LOC LOCATION TO TO CONDITION LONG SIGNER LONG SIGNER TO TO CONDITION LONG SIGNER		_	I/F	_ INSIDE FACE		
CEORE	C	_CAMBER		_		
CG	CANT	_	INTERM	_INTERMEDIATE		
CONSTRUCTION JOINT CONFIDENCE CONSTRUCTION JOINT COMPLETE JOINT PENETRATION COMPLETE JOINT PE		_				
CONSTRUCTION JOINT		_	JT	_JOINT		_
Compart		_	_			
CLIC			k	_KIPS		- ' '
CLEAR, CLEAR, ALCE CLEAR, CL		_				_
CLT		_	L			
COULD				_		
COLUMN LONG LEG HORIZONTAL STRUCT STRUCT STRUCT MERIAL CONC COMRESSION LUV LONG LEG VERTICAL SUPT SUPPORT(S) COND CONDRETE LOC LOCATION ToP TOP COND CONDROTONIOUS LSL LONG SIDE HORIZONTAL TEB OF TEB TOP AND BOTTOM CONX CONNECTION(S) LSL LAMINATED STRAND LUMBER TCA XLD TOP CHORD AXIAL LOD CONX CONNECTION(S) LSL LAMINATED STRAND LUMBER TCX TOP CHORD AXIAL LOD CCOVER PLATE LSV LAMINATED STRAND LUMBER TCX TOP CHORD AXIAL LOD CSINK COUNTERSINK MARCHANDER TTX TO CHORD AXIAL LOD CSINK COUNTERSINK MARCHANDER TTX TTX TO CHORD EXTENSIO CTOY CONTROL JOINT MAIL MANIHAMINA TTRAND THRAD THRAD <t< td=""><td></td><td>_</td><td></td><td>_</td><td></td><td></td></t<>		_		_		
COM COMPRESSION LLV LONG LEG VERTICAL SUPT SUPPORTIS) CONC CONCECTE LOC LOCATION TOP TOP TOP COND CONDITION LONGT LONGTUDINAL T TOP TOP TOP TOP TOP TOP AND BOTTOM TOP TOP AND BOTTOM TOP TOP AND BOTTOM TOP TOP AND BOTTOM TOP TOP AND BOTTOM						
CONCETE LOC _ LOCATION LOCATION T _ TOP COND CONDITION LONGIT LONGITUDINAL T _ TOP OF CONSTRUCTION LP _ LOW POINT T/ TOP OF CONT _ CONTRUOUS LSH _ LONG SIDE HORIZONTAL TAB OF TAB OF AND BOTTOM CONX _ CONNECTION(S) LSL _ LAMINATED STRAND LUMBER TC AX LD _ TOP CHORD BARBING CP _ COVER PLATE LVL _ LAMINATED STRAND LUMBER TC AX LD _ TOP CHORD BARBING CC CRITICAL ROOT ZONE LVL _ LAMINATED STRAND LUMBER TC CRITICAL ROOT ZONE CSINK _ COUNTERSINK M _ MOMENT THK _ THICKNES) CTJ _ CONTROL JOINT M _ M _ MOMENT M _ MOMENT THK _ THICKNES) CTD _ CONTROL JOINT M _ M _ MATEMAL		_		_		
COND CONDITION LONGT LONGTUDINAL T TOP CONST CONSTRUCTION LP LOW POINT 17. TOP OF TOP AND BOTTOM CONX CONNECTION(S) LSL LAMINATED STRAND LUMBER TCA AV LD TOP CHORD AXIAL LOX COORD COORDINATE LV LONG SIGE VERTICAL TCB TOP CHORD AXIAL LOX CRZ CRITICAL ROOT ZONE LWT LIGHTWEIGHT TCS TOP CHORD AXIAL DOX CRIZ CRITICAL ROOT ZONE LWT LIGHTWEIGHT TCS TOP CHORD AXIAL DOX CTD CONTROL JOINT M MOMENT THK THICK, THICK, MESS CTD CONTREDIO MAX MAXIMUM THK THICK, THICK, MESS DER DEMO DREGERBE BAR ANCHORIS MECH MECHANICAL TTYP TYPICAL DEBD DOUBLE MECH MECHANICAL TYP TYPICAL DEMOUSH MER MANURAL MARIANUM TTYP TYPICAL DEMOUSH MER MANURAL CATURE(R) UBC UN		_		_	2011	_5UPPOKT(5)
CONSTRUCTION LP LOW POINT T/TOP OF CONT CONTINUOUS LSL LONG SIGE HORIZONTAL TAB OF TB. TOP AND BOTTOM COND COONECTION(S) LSL LAMINATED STRAND LUMBER TC AX LD TOP CHORD BARING COORD COORDINATE LSV LONG SIGE VERTICAL TC AX LD TOP CHORD BARING CP COVER PLATE LVL LAMINATED VENEER LUMBER TCX TOP CHORD BARING CSINK COUNTERSINK CTTD CONTROL JOINT M MOMENT THK THK THK THK THCKINS SYSTEM CTID CONTROL JOINT M MOMENT THRD THREADED THREADED CTID MATL MATL MATL MATL THREADED THREADED CTID CONTROL MATL MATL MATL THREADED THREADED DAD DECONDERD MEZZAMINE MEZZAMINE THRO THROS TRANSVERSE DBL DEGONISH MEZZAMINE MEZZAMINE UNU UNIVERNIFORCED MARINE V				_	т	TOP
CONT CONT NUOUS LSH LONG SIDE HORIZONTAL TAB 6 or TB TOP AND BOTTON COORD COOND CONNECTION(S) LSL LAMINATED STRAND LUMBER T.CA X LD. TOP CHORD AXIAL LOX COORD COORDINATE LV LONG SIDE VERTICAL TCB TOP CHORD BATRAIN CRZ CRITICAL ROOT ZONE LWT LIGHTWEIGHT TCS TOP CHORD BATRAIN CRZ CRITICAL ROOT ZONE LWT LIGHTWEIGHT TCS TOP CHORD BATRAIN CTD CONTROL JOINT M MOMENT THK THKCKINESS CTD CONTROL JOINT M MOMENT THK THKCKINESS CTD CONTRELL MECH MAX MAXIMUM TPG TOPPING DER DEGREE BARRANCHORISS MECH MECHANICAL TPG TYPCAL TYPCAL DED DOUBLE MECH MECHANICAL TYPCAL TYPCAL DEMOULBH MFR MANURAL CAPURER UNIFORM BUILDING CO TYPCAL DEMOULBE BAR ANCHORISS MR MINIMIMUM UNIN		_		_		_
CONX CONNECTION(S) LSL LAMINATED STRAND LUMBER (CORD) T.C. X.L.D. TOP CHORD ASIAL LOA CORDINATE COORD COORDINATE LSV LONG SIDE VERTICAL T.C.B. TOP CHORD BEARING CTZ CRITCAL ROOT ZONE LVL LAMINATED VENEER LUMBER T.C.X. T.C.S. TOP CHORD STRAND CSINK COUNTERSINK M.M. MATL				_		
COORDINATE		_				
CP COVER PLATE LVL LAMINATED VENEER LUMBER TCX TOP CHORD EXTRISION CSINK COUNTERSINK TL LIGHTWEIGHT TES TIE DOWN SYSTEM CTRD CENTRED MATL MATL MATL MATL THED THED THEG TOPPING DB BEAM DROPPED) BEAM MC MOMENT THE THED THERADED THED THERADED THED THERADED THED THERADED THEN THE COURT THED THERADED THEN THERADED THEN				_		
CRZ		_		_		_
CSINK		_		_		
CONTROL_JOINT		_				_
CIRTÉRED		_	M	MOMENT		_
DR BEAM		_				
DR BEAM DROPPED BEAM	-	_				-
DBAL DOUBLE DECH DOUBLE MECH MEZZ ANINE MECH MEZZ ANINE TYP CAL TYPICAL DEMO DUBLE MEZZ ANINE UBC UNIFORM BUILDING CC UMU UNREINFORCED MASO UNIFORM BUILDING CC DEWO DEV DEVELOPMENT MID	DR BEAM	_DROP(PED) BEAM		_		
DBL DOUBLE MEZZ MEZZANINE MEZZANINE DEWO DEWOLISH MFR MANUFACTURE(R) UBC UNIFORM BUILDING CC DEV DEVELOPMENT MID MID MID UND UNREINFORCED MASO DIAG DIAGONAL MISC MISC MISCELLANEOUS UR UNDERREAM DIM DIMENSION(S) MTL METAL V SHEAR FORCE DIST DISTRIBUTED V SHEAR FORCE VERT VERTIFICAL DL DEAD LOAD NF NEAR FACE VERT VERTIFICAL DN DOWN NIC NOT IN CONTRACT VIF VERTIFICAL DP DEPTH/DEEP NIT NAIL AMINATED TIMBER VOL VOLUME DTL DETALIS(S)D) NO NUMBER VOL VOLUME DWG DRAWING(S) NOM NOMINAR W WIDE DWG DOWL(S) NS NON-SHRINK W/ WITHOUT EA ACCH FACE <t< td=""><td></td><td></td><td></td><td></td><td></td><td></td></t<>						
DEVELOPMENT MID				-		
DIR	DEMO	_		-		_UNIFORM BUILDING CODI
DIAGONAL MISC MISCELLANEOUS UR UNDER-REAM	DEV	_		_		_ _UNREINFORCED MASONR
DIMENSION(S)		_		_		_UNLESS NOTED OTHERWI
DISTRIBUTED		_		-	UR	_UNDER-REAM
DEAD LOAD			MTL	_METAL		
DN		_				_
DP DEPTHIDEEP NLT NAIL LAMINATED TIMBER VOL VOLUME DTL DETAIL(S)(D) No NUMBER NUMBER W WIDE DWG DRAWING(S) NOM NOM NOMINAL W WIDE DWL DOWEL(S) NS NON-SHRINK W WITH DWL DOWEL(S) NS NON-SHRINK W WID LOAD EA EACH NW NORMAL WEIGHT WHS WID LOAD EE EXPANSION JOINT OC ON CENTER W/D WITHOUT EL ELEVATION OD OUTSIDE DIAMETER WP WORK POINT ELEC ELECTRICAL O/F OUTSIDE FACE WPR WATER PROOFING ELEV ELECTRICAL O/F OUTSIDE FACE WPR WATER PROOFING ELEV ELECTRICAL O/F OUTSIDE FACE WPR WATER PROOFING ELEC ELECTRICAL O/F OUTSIDE FACE WPR WATER PROOFING ELEC		_		_		_
DTL DETAIL(S)(D) No NUMBER DWG DRAWING(S) NOM NOMINAL W WIDE DWL DOWEL(S) NS NON-SHRINK W///>WITH WITH WIT		_		_		_
DWG		_		_	VOL	_VOLUME
DOWEL(S) NS				_		
NTS		_		_		_
EA EACH FACE NW NORMAL WEIGHT WHS WELDED HEADED STUE EF EACH FACE WL WIND LOAD EJ EXPANSION JOINT OC ON CENTER WO WITHOUT EL ELEVATION OD OUTSIDE DIAMETER WP WORK POINT ELEC ELECTRICAL O/F OUTSIDE FACE WPR WATER PROOFING ELEV ELEVATOR OH OPPOSITE HAND WS WATER STOP EMBED EMBEDMENT OPNG OPPOSITE WWF WELDED WIRE FABRIC ENG ENGINEER OF RECORD OSB ORIENTED STRAND BOARD XS EXTRA STRONG EQ EOUAL OVHG OVERHANG XXS EXTRA STRONG EQ EOUAL OVHG OVERHANG XXS EXTRA EXTRA STRONG EQPT EQUAL OVHG OVERHANG XXS EXTRA EXTRA STRONG EQPT EQUAL OVHG OVERHANG XXS EXTRA EXTRA STRONG EQPT EXPANSION	DWL	_DOWEL(S)				
EF EACH FACE WL WIND LOAD EJ EXPANSION JOINT OC ON CENTER W/O WITHOUT EL ELEVATION OD OUTSIDE DIAMETER WP WORK POINT ELEC ELEVATOR OH OPPOSITE WS WATER STOP EMBED EMBEDMENT OPNG OPENING(S) WWF WELDED WIRE FABRIC ENGINEER(D) OPP OPPOSITE WWF WELDED WIRE FABRIC ENG ENGINEER(D) OPP OPPOSITE WWF WELDED WIRE FABRIC EOR ENGINEER OF RECORD OSB ORIENTED STRAND BOARD XS EXTRA STRONG EOR ENGINEER OF RECORD OSB ORIENTED STRAND BOARD XS EXTRA STRONG EQ EQUIPMENT OWSJ OPEN WEB STEEL JOIST XXS EXTRA STRONG EQPT EQUIPMENT OWSJ OPEN WEB WOOD JOIST EXTRA EXTRA STRONG EXIST EXISTING PAF POWDER ACTUALED FASTENER PARTIAL JOINT EXY EXTEROLOR	Γ Λ	FACIL		_		
EXPANSION JOINT			NW	_NORMAL WEIGHT		_
ELEC ELECTRICAL O/F OUTSIDE DIAMETER WP WORK POINT ELEC ELECTRICAL O/F OUTSIDE FACE WPR WATER PROOFING ELEV ELEVATOR OH OPPOSITE HAND WS WATER STOP EMBED EMBEDMENT OPNG OPENING(S) WWF WELDED WIRE FABRIC ENG ENGINEER(D) OPP OPPOSITE EOR ENGINEER OF RECORD OSB ORIENTED STRAND BOARD XS EXTRA STRONG EQ EQUAL OVHG OVERHANG XXS EXTRA STRONG EQPT EQUIPMENT OWSJ OPEN WEB STEEL JOIST EW EACH WAY OWNJ OPEN WEB WOOD JOIST EXIST EXISTING EXP EXPANSION P AXIAL LOAD EXT EXTERIOR PAF POWDER ACTUATED FASTENER PC PIER/PILE CAP F/C PRECAST CONCRETE FAB FABRICATE(ION)(OR) PCF POUNDS PER CUBIC FOOT FD FLOOR DRAIN PCY POUNDS PER CUBIC YARD FF FINISHED FLOOR PERP PERPENDICULAR FLG FLANGE PJP PARTIAL JOINT PENETRATION FIND FOUNDATION PL FF FIREPROOF(ING) PLF POUNDS PER LINEAR FOOT FRM FRAMING PLYWOOD FRT FIRE RETARDANT TREATED PREFAB PREFABRICATED FS FAR SIDE PRELIM PRELIMINARY FSTN FASTEN(ED) PSF POUNDS PER SQUARE FOOT FTF FIRE RETARDANT TREATED PREFAB PREFABRICATED FS FAR SIDE PRELIM PRELIMINARY FSTN FASTEN(ER)(ED) PSF POUNDS PER SQUARE FOOT FTF FACE TO FACE PSI POUNDS PER SQUARE INCH FTF FACE TO FACE PSI POUNDS PER SQUARE INCH FTF FACE TO FACE PSI POUNDS PER SQUARE INCH FTF FACE TO FACE PSI POUNDS PER SQUARE INCH FTF FACE TO FACE PSI POUNDS PER SQUARE INCH FTF FACE TO FACE PSI POUNDS PER SQUARE INCH FTF FACE TO FACE PSI POUNDS PER SQUARE INCH FTF FACE TO FACE PSI POUNDS PER SQUARE INCH FTF FACE TO FACE PSI POUNDS PER SQUARE INCH FTF FACE TO FACE PSI POUNDS PER SQUARE INCH FTF FACE TO FACE PSI POUNDS PER SQUARE INCH FTF PRESSURE TREATED			00	ON CENTED		
ELEC ELECTRICAL O/F OUTSIDE FACE WPR WATER PROOFING ELEV ELEVATOR OH OPPOSITE HAND WS WATER STOP EMBED EMBED EMBEDMENT OPNG OPENING(S) WWF WELDED WIRE FABRIC ENGINEER(D) OPP OPPOSITE ENG ENGINEER OF RECORD OSB ORIENTED STRAND BOARD XS EXTRA STRONG EQ EQUAL OVHG OVERHANG XXS EXTRA STRONG EQPT EQUIPMENT OWSJ OPEN WEB STEEL JOIST OWSJ OPEN WEB WOOD JOIST EXISTING EXPENSION PAFF POWDER ACTUATED FASTENER PC PIER/PILE CAP PC POUNDS PER CUBIC FOOT FD FLOOR DRAIN PCY POUNDS PER CUBIC FOOT FD FLOOR PERP PERPENDICULAR FLOOR PERP PERPENDICULAR FLOOR PERP PERPENDICULAR FLOOR PERP PERPENDICULAR FLOOR PLATE FLOOR PERP PERPENDICULAR FOUNDS PER LINEAR FOOT PLATE FLOOR PLATE FLOOR PLATE FLOOR PLATE FLOOR PLATE FLOOR PLATE FLOOR PERP PERPENDICULAR FOOT PLATE FLOOR PLATE FLOOR PLATE FLOOR PLATE FLOOR PLATE FLOOR PLATE FLOOR PERP PERPENDICULAR FOOT PLATE FLOOR PLATE		_				
ELEV ELEVATOR OH OPPOSITE HAND WS WATER STOP EMBED EMBEDMENT OPNG OPENING(S) WWF WELDED WIRE FABRIC ENG ENGINEER(D) OPP OPPOSITE EOR ENGINEER OF RECORD OSB ORIENTED STRAND BOARD XS EXTRA STRONG EQ EQUAL OVHG OVERHANG XXS EXTRA EXTRA STRONG EQPT EQUIPMENT OWSJ OPEN WEB STEEL JOIST EW EACH WAY OWJ OPEN WEB WOOD JOIST EXIST EXISTING EXP EXPANSION P AXIAL LOAD EXT EXTERIOR PAF POWDER ACTUATED FASTENER FC PIER/PILE CAP F/ FACE OF P/C PRECAST CONCRETE FAB FABRICATE(ION)(OR) PCF POUNDS PER CUBIC FOOT FD FLOOR DRAIN PCY POUNDS PER CUBIC YARD FF FINISHED FLOOR PEN PERFORATED FL FLOOR PERP PERFORATED FL FLOOR PERP PERFORATED FL FLOOR PERP PARTIAL JOINT PENETRATION FIND FOUNDATION PL PLATE FP FIREPROOF(ING) PLF POUNDS PER LINEAR FOOT FRM FRAMING PLYWO D FRT FIRE RETARDANT TREATED PREFAB PREFABRICATED FS FAR SIDE PRELIM PRELIMINARY FSTN FASTEN(ER)(ED) PSF POUNDS PER SQUARE FOOT FTF FACE TO FACE PSI POUNDS PER SQUARE FOOT FTF FACE TO FACE PSI POUNDS PER SQUARE INCH FTF FACE TO FACE PSI POUNDS PER SQUARE INCH FTF FACE TO FACE PSI POUNDS PER SQUARE INCH FTF FACE TO FACE PSI POUNDS PER SQUARE INCH FTF FACE TO FACE PSI POUNDS PER SQUARE INCH FTF FACE TO FACE PSI POUNDS PER SQUARE INCH FTF FREE PERFORE PSI POUNDS PER SQUARE INCH FTF FREE FEACE PSI POUNDS PER SQUARE INCH FTF FACE TO FACE PSI POUNDS PER SQUARE INCH FTF FACE TO FACE PSI POUNDS PER SQUARE INCH FTF PRESSURE TREATED		_		_		_
EMBED EMBEDMENT OPNG OPENING(S) WWF WELDED WIRE FABRIC ENG ENGINEER(D) OPP OPPOSITE ENGINEER OF RECORD OSB ORIENTED STRAND BOARD XS EXTRA STRONG EQ EQUAL OVHG OVERHANG XXS EXTRA EXTRA STRONG EQPT EQUIPMENT OWSJ OPEN WEB STEEL JOIST EW EACH WAY OWUJ OPEN WEB WOOD JOIST EXIST EXISTING EXPANSION P AXIAL LOAD EXY EXTERIOR PAF POWDER ACTUATED FASTENER EXT EXTERIOR PAF POWDER ACTUATED FASTENER FC PIER/PILE CAP PC PIER/PILE CAP F/_ FACE OF P/C PRECAST CONCRETE FAB FABRICATE(ION)(OR) PCF POUNDS PER CUBIC FOOT FD FLOOR DRAIN PCY POUNDS PER CUBIC YARD FF FINISHED FLOOR PEN PERFEDRATION FIL FLOOR PERF PERFEDRATION FIL FLOOR <				_		
ENG ENGINEER(D) OPP OPPOSITE EOR ENGINEER OF RECORD OSB ORIENTED STRAND BOARD XS EXTRA STRONG EQ EQUAL OVHG OVERHANG XXS EXTRA EXTRA STRONG EQPT EQUIPMENT OWSJ OPEN WEB STEEL JOIST EW EACH WAY OWWJ OPEN WEB WOOD JOIST EXISTING EXISTING EXPANSION P AXIAL LOAD EXP EXPANSION P AXIAL LOAD EXPANSION P AXIAL LOAD EXT EXTERIOR PAF POWDER ACTUATED FASTENER POWDER SET CUATE FASTENER POWDER SET CUATE F						
EOR ENGINEER OF RECORD OSB ORIENTED STRAND BOARD XS EXTRA STRONG EQ EQUAL OVHG OVERHANG XXS EXTRA EXTRA STRONG EQPT EQUIPMENT OWSJ OPEN WBS STEEL JOIST EW EACH WAY OWJ OPEN WEB WOOD JOIST EXIST EXISTING EXP EXPANSION P AXIAL LOAD EXT EXTERIOR PAF POWDER ACTUATED FASTENER EXP FACE OF PC PIER/PILE CAP F/ PRECAST CONCRETE FAB FABRICATE(ION)(OR) PCF POUNDS PER CUBIC FOOT FD FLOOR DRAIN PCY POUNDS PER CUBIC YARD FF FINISHED FLOOR PEN PERPENDICULAR FIN FINISH(ED) PERF PERPENDICULAR FLG FLANGE PJP PARTIAL JOINT PENETRATION FND FOUNDATION PL PLATE FP FIREPROOF(ING) PLF POUNDS PER LINEAR FOOT FRM FRAMING PLYWD PLYWOOD FRT FIRE RETARDANT TREATED PREFAB PREFABRICATED FS FAR SIDE PRELIM PRELIMINARY FSTN FASTEN(ER)(ED) PSF POUNDS PER SQUARE FOOT FTF FACE TO FACE PSI PARALLEL STRAND LUMBER FTG FOOTING PSL PARALLEL STRAND LUMBER FTG FOOTING PSL PARALLEL STRAND LUMBER FTG FOOTING PSL PARALLEL STRAND LUMBER FTG PRESSURE TREATED		_			V V V V F	-AAFFDFD MIKE LARKIC
EQ EQUAL OVHG OVERHANG XXS EXTRA EXTRA STRONG EQPT EQUIPMENT OWSJ OPEN WEB STEEL JOIST EW EACH WAY OWUJ OPEN WEB WOOD JOIST EXIST EXISTING EXPANSION P AXIAL LOAD EXT EXTERIOR PAF POWDER ACTUATED FASTENER EXT EXTERIOR PAF POWDER ACTUATED FASTENER FC PIER/PILE CAP PERPLIE CAP F/ FACE OF P/C PRECAST CONCRETE FAB FABRICATE(ION)(OR) PCF POUNDS PER CUBIC FOOT FD FLOOR DRAIN PCF POUNDS PER CUBIC YARD FF FINISHED FLOOR PEN PENETRATION FIN FINISHED FLOOR PERP PERFORATED FL FLOOR PERF PERFORATED FL FLOOR PERP PERFORATED FL FLOOR PERP PERPENDICULAR FLG FLANGE PJP PARTIAL JOINT PENETRATION FND		_ ` ` `		-	XS	EXTRA STRONG
EQPT EQUIPMENT OWSJ OPEN WEB STEEL JOIST EW EACH WAY OWWJ OPEN WEB WOOD JOIST EXIST EXISTING EXP EXPANSION P AXIAL LOAD EXT EXTERIOR PAF POWDER ACTUATED FASTENER PC PIER/PILE CAP F/ FACE OF P/C PRECAST CONCRETE FAB FABRICATE(ION)(OR) PCF POUNDS PER CUBIC FOOT FD FLOOR DRAIN PCY POUNDS PER CUBIC YARD FF FINISHED FLOOR PEN PENETRATION FIN FINISH(ED) PERF PERFORATED FL FLOOR PERP PERPENDICULAR FLG FLANGE PJP PARTIAL JOINT PENETRATION FND FOUNDATION PL PLATE FP FIREPROOF(ING) PLF POUNDS PER LINEAR FOOT FRM FRAMING PLYWD PLYWOOD FRT FIRE RETARDANT TREATED PREFAB PREFABRICATED FS FAR SIDE PRELIM PRELIMINARY FSTN FASTEN(ER)(ED) PSF POUNDS PER SQUARE FOOT FTF FACE TO FACE PSI PORNS PER SQUARE INCH FTG FOOTING PSL PARALLEL STRAND LUMBER PT PRESSURE TREATED				_		_
EW EACH WAY OWWJ OPEN WEB WOOD JOIST EXIST EXISTING EXP EXPANSION P AXIAL LOAD EXT EXTERIOR PAF POWDER ACTUATED FASTENER PC PIER/PILE CAP F/ FACE OF P/C PRECAST CONCRETE FAB FABRICATE(ION)(OR) PCF POUNDS PER CUBIC FOOT FD FLOOR DRAIN PCY POUNDS PER CUBIC YARD FF FINISHED FLOOR PEN PENETRATION FIN FINISH(ED) PERF PERFORATED FL FLOOR PERP PERPENDICULAR FLG FLANGE PJP PARTIAL JOINT PENETRATION FND FOUNDATION PL PLATE FP FIREPROOF(ING) PLF POUNDS PER LINEAR FOOT FRM FRAMING PLYWD PLYWOOD FRT FIRE RETARDANT TREATED PREFAB PREFABRICATED FS FAR SIDE PRELIM PRELIMINARY FSTN FASTEN(ER)(ED) PSF POUNDS PER SQUARE FOOT FTF FACE TO FACE PSI POUNDS PER SQUARE INCH FTG FOOTING PSL PARALLEL STRAND LUMBER PT PRESSURE TREATED						
EXIST EXISTING EXP EXPANSION P AXIAL LOAD EXT EXTERIOR PAF POWDER ACTUATED FASTENER PC PIER/PILE CAP F/ FACE OF P/C PRECAST CONCRETE FAB FABRICATE(ION)(OR) PCF POUNDS PER CUBIC FOOT FD FLOOR DRAIN PCY POUNDS PER CUBIC YARD FF FINISHED FLOOR PEN PENETRATION FIN FINISH(ED) PERF PERFORATED FL FLOOR PERP PERPENDICULAR FLG FLANGE PJP PARTIAL JOINT PENETRATION FND FOUNDATION PL PLATE FP FIREPROOF(ING) PLF POUNDS PER LINEAR FOOT FRM FRAMING PLYWD PLYWOOD FRT FIRE RETARDANT TREATED PREFAB PREFABRICATED FS FAR SIDE PRELIMINARY FSTN FASTEN(ER)(ED) PSF POUNDS PER SQUARE FOOT FTF FACE TO FACE PSI POUNDS PER SQUARE INCH FTG FOOTING PSL PARSURE TREATED				-		
EXP EXPANSION P AXIAL LOAD EXT EXTERIOR PAF POWDER ACTUATED FASTENER PC PIER/PILE CAP F/ FACE OF P/C PRECAST CONCRETE FAB FABRICATE(ION)(OR) PCF POUNDS PER CUBIC FOOT FD FLOOR DRAIN PCY POUNDS PER CUBIC YARD FF FINISHED FLOOR PEN PERFORATED FIN FINISH(ED) PERF PERFORATED FL FLOOR PERP PERPENDICULAR FLG FLANGE PJP PARTIAL JOINT PENETRATION FND FOUNDATION PL PLATE FP FIREPROOF(ING) PLF POUNDS PER LINEAR FOOT FRM FRAMING PLYWD PLYWOOD FRT FIRE RETARDANT TREATED PREFAB PREFABRICATED FS FAR SIDE PRELIM PRELIMINARY FSTN FASTEN(ER)(ED) PSF POUNDS PER SQUARE FOOT FTF FACE TO FACE PSI PORNDE PRESSURE TREATED						
EXT EXTERIOR PAF POWDER ACTUATED FASTENER PC PIER/PILE CAP F/ FACE OF P/C PRECAST CONCRETE FAB FABRICATE(ION)(OR) PCF POUNDS PER CUBIC FOOT FD FLOOR DRAIN PCY POUNDS PER CUBIC YARD FF FINISHED FLOOR PEN PENETRATION FIN FINISH(ED) PERF PERFORATED FL FLOOR PERP PERPENDICULAR FLG FLANGE PJP PARTIAL JOINT PENETRATION FND FOUNDATION PL PLATE FP FIREPROOF(ING) PLF POUNDS PER LINEAR FOOT FRM FRAMING PLYWD PLYWOOD FRT FIRE RETARDANT TREATED PREFAB PREFABRICATED FS FAR SIDE PRELIM PRELIMINARY FSTN FASTEN(ER)(ED) PSF POUNDS PER SQUARE FOOT FTF FACE TO FACE PSI POUNDS PER SQUARE INCH FTG FOOTING PSL PARALLEL STRAND LUMBER FT PRESSURE TREATED			Р	AXIAL LOAD		
F/_ FACE OF P/C PRECAST CONCRETE FAB FABRICATE(ION)(OR) PCF POUNDS PER CUBIC FOOT FD FLOOR DRAIN PCY POUNDS PER CUBIC YARD FF FINISHED FLOOR PEN PENETRATION FIN FINISH(ED) PERF PERFORATED FL FLOOR PERP PERPENDICULAR FLG FLANGE PJP PARTIAL JOINT PENETRATION FND FOUNDATION PL PLATE FP FIREPROOF(ING) PLF POUNDS PER LINEAR FOOT FRM FRAMING PLYWO PLYWOOD FRT FIRE RETARDANT TREATED PREFAB PREFABRICATED FS FAR SIDE PRELIM PRELIMINARY FSTN FASTEN(ER)(ED) PSF POUNDS PER SQUARE FOOT FTF FACE TO FACE PSI PARALLEL STRAND LUMBER FT PRESSURE TREATED		_		_		
F/FACE OF		_		_		
FAB FABRICATE(ION)(OR) PCF POUNDS PER CUBIC FOOT FD FLOOR DRAIN PCY POUNDS PER CUBIC YARD FF FINISHED FLOOR PEN PENETRATION FIN FINISH(ED) PERF PERFORATED FL FLOOR PERP PERPENDICULAR FLG FLANGE PJP PARTIAL JOINT PENETRATION FND FOUNDATION PL PLATE FP FIREPROOF(ING) PLF POUNDS PER LINEAR FOOT FRM FRAMING PLYWD PLYWOOD FRT FIRE RETARDANT TREATED PREFAB PREFABRICATED FS FAR SIDE PRELIM PRELIMINARY FSTN FASTEN(ER)(ED) PSF POUNDS PER SQUARE FOOT FTF FACE TO FACE PSI POUNDS PER SQUARE INCH FTG FOOTING PSL PARALLEL STRAND LUMBER PT PRESSURE TREATED	F/	_FACE OF				
FD FLOOR DRAIN PCY POUNDS PER CUBIC YARD FF FINISHED FLOOR PEN PENETRATION FIN FINISH(ED) PERF PERFORATED FL FLOOR PERP PERPENDICULAR FLG FLANGE PJP PARTIAL JOINT PENETRATION FND FOUNDATION PL PLATE FP FIREPROOF(ING) PLF POUNDS PER LINEAR FOOT FRM FRAMING PLYWD PLYWOOD FRT FIRE RETARDANT TREATED PREFAB PREFABRICATED FS FAR SIDE PRELIM PRELIMINARY FSTN FASTEN(ER)(ED) PSF POUNDS PER SQUARE FOOT FTF FACE TO FACE PSI POUNDS PER SQUARE INCH FTG FOOTING PSL PARALLEL STRAND LUMBER PT PRESSURE TREATED				_		
FF FINISHED FLOOR PEN PENETRATION FIN FINISH(ED) PERF PERFORATED FL FLOOR PERP PERPENDICULAR FLG FLANGE PJP PARTIAL JOINT PENETRATION FND FOUNDATION PL PLATE FP FIREPROOF(ING) PLF POUNDS PER LINEAR FOOT FRM FRAMING PLYWD PLYWOOD FRT FIRE RETARDANT TREATED PREFAB PREFABRICATED FS FAR SIDE PRELIM PRELIMINARY FSTN FASTEN(ER)(ED) PSF POUNDS PER SQUARE FOOT FTF FACE TO FACE PSI POUNDS PER SQUARE INCH FTG FOOTING PSL PARALLEL STRAND LUMBER PT PRESSURE TREATED	-	_		-		
FL FLOOR PERP PERPENDICULAR FLG FLANGE PJP PARTIAL JOINT PENETRATION FND FOUNDATION PL PLATE FP FIREPROOF(ING) PLF POUNDS PER LINEAR FOOT FRM FRAMING PLYWD PLYWOOD FRT FIRE RETARDANT TREATED PREFAB PREFABRICATED FS FAR SIDE PRELIM PRELIMINARY FSTN FASTEN(ER)(ED) PSF POUNDS PER SQUARE FOOT FTF FACE TO FACE PSI POUNDS PER SQUARE INCH FTG FOOTING PSL PARALLEL STRAND LUMBER FT PRESSURE TREATED		_FINISHED FLOOR		_		
FL FLOOR PERP PERPENDICULAR FLG FLANGE PJP PARTIAL JOINT PENETRATION FND FOUNDATION PL PLATE FP FIREPROOF(ING) PLF POUNDS PER LINEAR FOOT FRM FRAMING PLYWD PLYWOOD FRT FIRE RETARDANT TREATED PREFAB PREFABRICATED FS FAR SIDE PRELIM PRELIMINARY FSTN FASTEN(ER)(ED) PSF POUNDS PER SQUARE FOOT FTF FACE TO FACE PSI POUNDS PER SQUARE INCH FTG FOOTING PSL PARALLEL STRAND LUMBER FT PRESSURE TREATED		_				
FND FOUNDATION PL PLATE FP FIREPROOF(ING) PLF POUNDS PER LINEAR FOOT FRM FRAMING PLYWD PLYWOOD FRT FIRE RETARDANT TREATED PREFAB PREFABRICATED FS FAR SIDE PRELIM PRELIMINARY FSTN FASTEN(ER)(ED) PSF POUNDS PER SQUARE FOOT FTF FACE TO FACE PSI POUNDS PER SQUARE INCH FTG FOOTING PSL PARALLEL STRAND LUMBER PT PRESSURE TREATED		= ` ',		_		
FPFIREPROOF(ING) PLFPOUNDS PER LINEAR FOOT FRMFRAMING PLYWDPLYWOOD FRTFIRE RETARDANT TREATED PREFABPREFABRICATED FSFAR SIDE PRELIMPRELIMINARY FSTNFASTEN(ER)(ED) PSFPOUNDS PER SQUARE FOOT FTFFACE TO FACE PSIPOUNDS PER SQUARE INCH FTGFOOTING PSLPARALLEL STRAND LUMBER PTPRESSURE TREATED		_FLANGE	PJP	PARTIAL JOINT PENETRATION		
FRM FRAMING PLYWD PLYWOOD FRT FIRE RETARDANT TREATED PREFAB PREFABRICATED FS FAR SIDE PRELIM PRELIMINARY FSTN FASTEN(ER)(ED) PSF POUNDS PER SQUARE FOOT FTF FACE TO FACE PSI POUNDS PER SQUARE INCH FTG FOOTING PSL PARALLEL STRAND LUMBER PT PRESSURE TREATED				_		
FRT FIRE RETARDANT TREATED PREFAB PREFABRICATED FS FAR SIDE PRELIM PRELIMINARY FSTN FASTEN(ER)(ED) PSF POUNDS PER SQUARE FOOT FTF FACE TO FACE PSI POUNDS PER SQUARE INCH FTG FOOTING PSL PARALLEL STRAND LUMBER PT PRESSURE TREATED		_				
FS FAR SIDE PRELIM PRELIMINARY FSTN FASTEN(ER)(ED) PSF POUNDS PER SQUARE FOOT FTF FACE TO FACE PSI POUNDS PER SQUARE INCH FTG FOOTING PSL PARALLEL STRAND LUMBER PT PRESSURE TREATED		FRAMING		_PLYWOOD		
FSTN FASTEN(ER)(ED) PSF POUNDS PER SQUARE FOOT FTF FACE TO FACE PSI POUNDS PER SQUARE INCH FTG FOOTING PSL PARALLEL STRAND LUMBER PT PRESSURE TREATED						
FTF FACE TO FACE PSI POUNDS PER SQUARE INCH FTG FOOTING PSL PARALLEL STRAND LUMBER PT PRESSURE TREATED				_		
FTGFOOTING PSLPARALLEL STRAND LUMBER PTPRESSURE TREATED						
PTPRESSURE TREATED				-		
	FTG	_FOOTING		_		
D.T. DOCT TENCIONIED				_		
P-TPOST-TENSION(ED)			P ₋ T	POST-TENSION(ED)		





SEAUX+PIERCE

1014 Sailmaster St. Austin, Texas 78734 www.seauxpierce.com

BURNET CITY HALL

ARCHITECT

300 East Jackson Street Burnet, Texas 78611

> **PERMIT SET** 05.19.2023

 4
 Addendum 2
 11.27.2023

 2
 NA
 NA

 NO
 ISSUE
 DATE

 PM: S. Tanner

ENG: P. El Hanna BIM PM: C. Hernandez QA/QC: S. Tanner

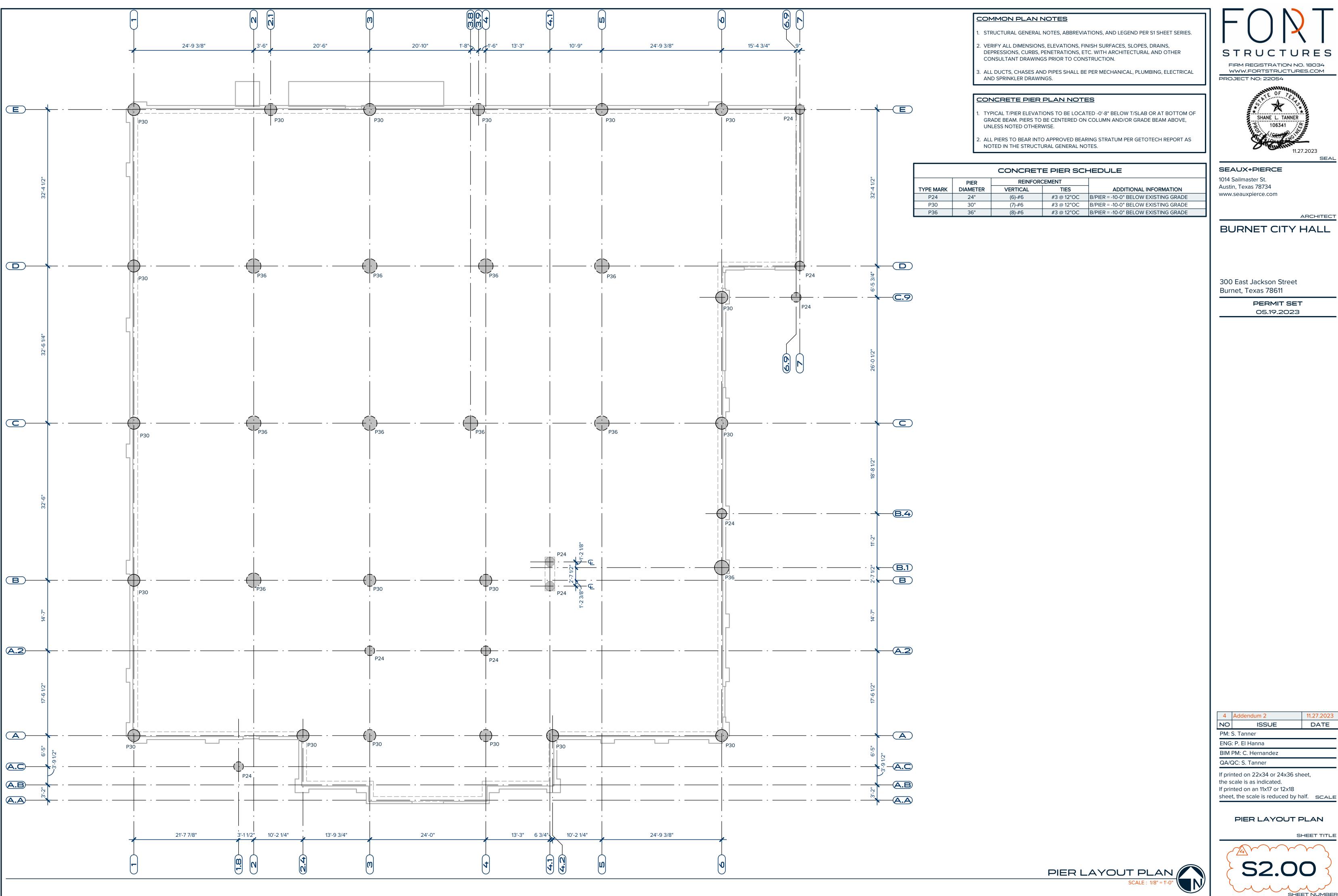
If printed on 22x34 or 24x36 sheet, the scale is as indicated.

If printed on an 11x17 or 12x18 sheet, the scale is reduced by half. SCALE

ABBREVIATIONS & LEGENDS

D.10

SHEET NUMBER



FIRM REGISTRATION NO. 18034



SEAUX+PIERCE 1014 Sailmaster St.

www.seauxpierce.com

BURNET CITY HALL

ARCHITECT

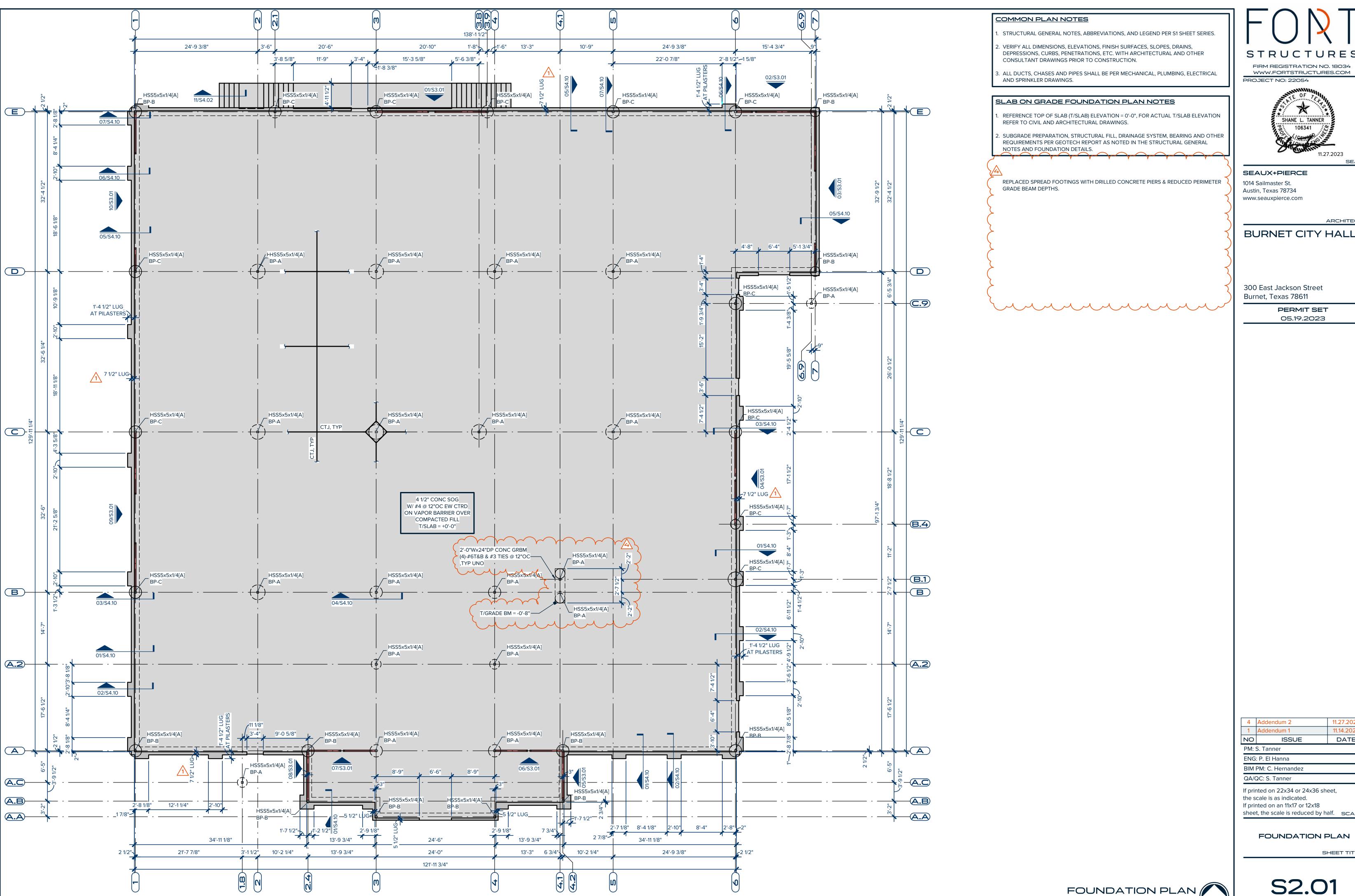
PERMIT SET 05.19.2023

11.27.2023 ISSUE DATE

If printed on 22x34 or 24x36 sheet, the scale is as indicated. If printed on an 11x17 or 12x18

PIER LAYOUT PLAN





FIRM REGISTRATION NO. 18034



SEAUX+PIERCE

1014 Sailmaster St. Austin, Texas 78734 www.seauxpierce.com

BURNET CITY HALL

ARCHITECT

300 East Jackson Street

PERMIT SET 05.19.2023

11.27.2023 4 Addendum 2 11.14.2023 Addendum 1 ISSUE DATE PM: S. Tanner

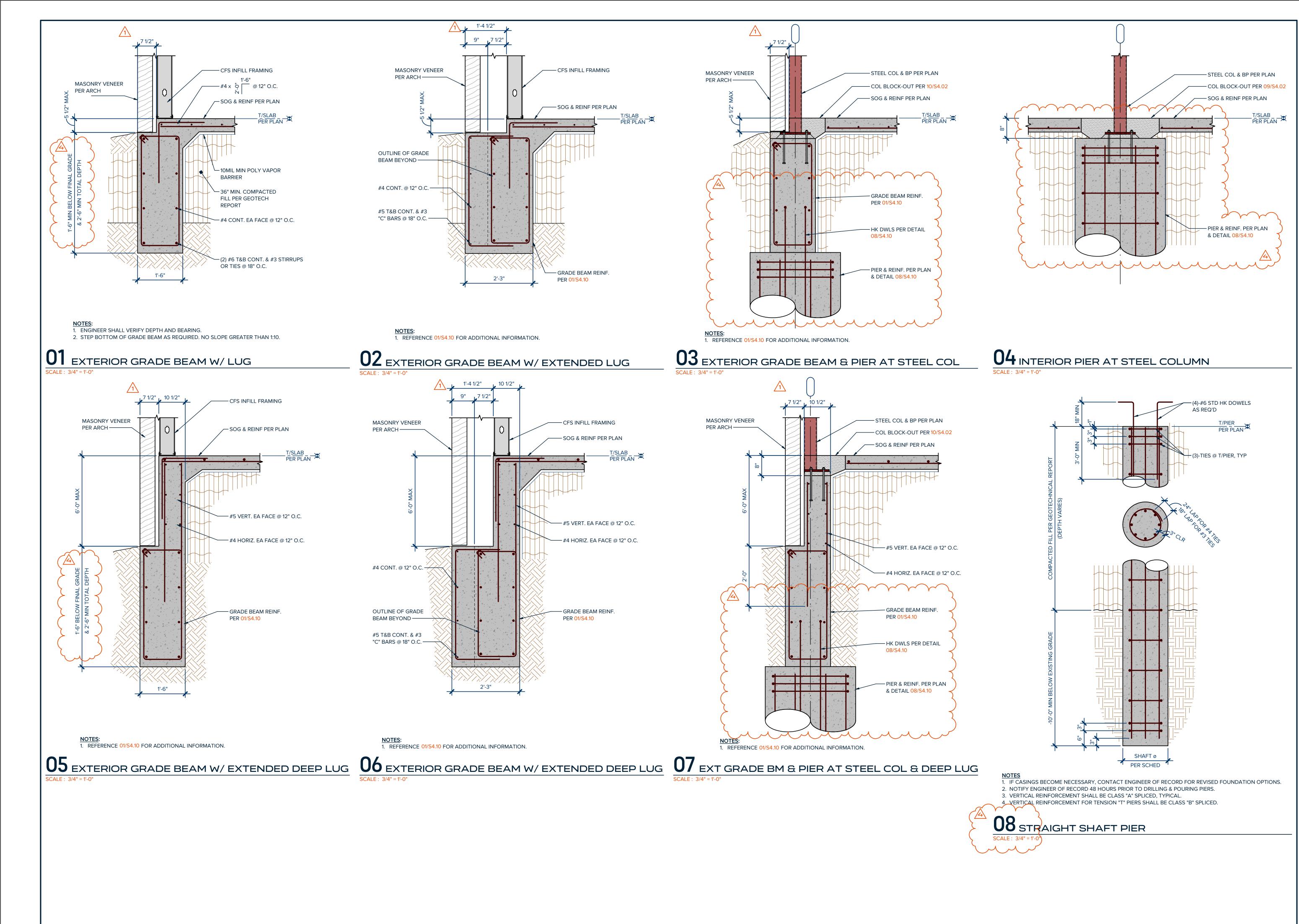
ENG: P. El Hanna

BIM PM: C. Hernandez QA/QC: S. Tanner

If printed on 22x34 or 24x36 sheet, the scale is as indicated. If printed on an 11x17 or 12x18 sheet, the scale is reduced by half. SCALE

FOUNDATION PLAN

S2.01



FIRM REGISTRATION NO. 18034
WWW.FORTSTRUCTURES.COM

SHANE L. TANNER
106341

SEAUX+PIERCE

1014 Sailmaster St.
Austin, Texas 78734
www.seauxpierce.com

BURNET CITY HALL

ARCHITECT

300 East Jackson Street Burnet, Texas 78611

> **PERMIT SET** 05.19.2023

4 Addendum 2 11.27.2023
1 Addendum 1 11.14.2023
NO ISSUE DATE
PM: S. Tanner

ENG: P. El Hanna BIM PM: C. Hernandez

COPYRIGHT © 2023 FORT STRUCTURES

If printed on 22x34 or 24x36 sheet, the scale is as indicated.

the scale is as indicated.

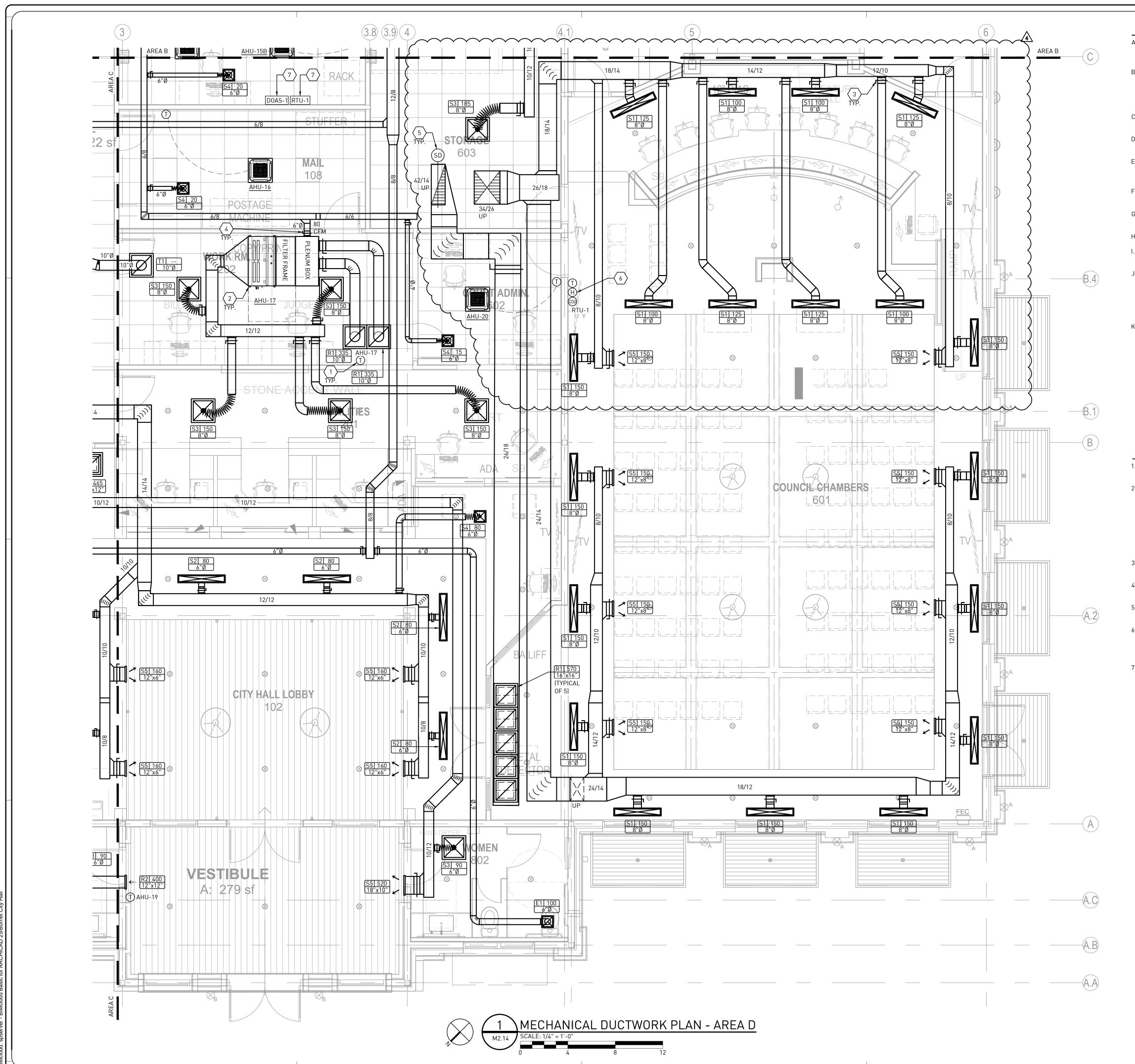
If printed on an 11x17 or 12x18

sheet, the scale is reduced by half.

FOUNDATION DETAILS

S4.10

SHEET TITLE



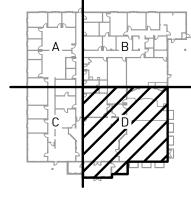
GENERAL NOTES

- A. THE CONTRACTOR SHALL REPLACE ALL FILTERS ON AHU'S AFTER CONSTRUCTION IS COMPLETE. UNDER NO CIRCUMSTANCES SHALL ANY EQUIPMENT BE OPERATED WITHOUT FILTERS INSTALLED. FILTER USED DURING CONSTRUCTION SHALL BE MINIMUM MERV 8.
- B. ALL TAPS AND DUCTWORK SERVING SUPPLY, RETURN, OUTSIDE AIR OR EXHAUST DIFFUSERS SHALL HAVE LOCKING BALANCING DAMPERS MATCHING THE DIFFUSER NECK DIAMETER UNLESS INDICATED OTHERWISE. POSITION BALANCING DAMPERS ABOVE CEILING WHENEVER FEASIBLE. WHERE DAMPER IS NOT ACCESSIBLE, PROVIDE CABLE-OPERATED ROTO-TWIST DAMPER, ACCESSIBLE FROM FACE OF DIFFUSER.
- C. FOR CLARITY PURPOSES, CEILING GRID, ALL EQUIPMENT, DUCTWORK, PIPING, ETC. MAY NOT BE SHOWN IN ALL VIEWS.
- D. DUCT SIZES LISTED ON PLANS ARE INSIDE FREE AIRWAY DIMENSIONS. FIRST FIGURE IN DUCT SIZE INDICATES THE DIMENSION OF THE FACE SHOWN OR INDICATED IN THE DRAWING VIEW.
- E. ACCESS PANELS IN INACCESSIBLE CEILINGS ARE REQUIRED FOR ALL VALVES, TRAPS, DAMPERS, CLEANOUTS, CONTROLS, ETC., AND SHALL BE FURNISHED AND INSTALLED UNDER ARCHITECTURAL SPECIFICATIONS.
- F. ALL 90° ELBOWS IN RECTANGULAR SUPPLY AND OUTSIDE AIR DUCTWORK SHALL HAVE TURNING VANES INSTALLED.
- G. CONTRACTOR SHALL COORDINATE DIFFUSER/GRILLE LOCATIONS WITH ARCHITECTURAL REFLECTED CEILING PLAN.
- H. MAXIMUM LENGTH OF FLEX DUCT SHALL NOT TO EXCEED 60 INCHES.
- I. CONTRACTOR SHALL FIELD VERIFY EXACT LOCATION OF <u>ALL</u> NEW EQUIPMENT AND COORDINATE WITH THE GENERAL CONTRACTOR.
- J. CONTRACTOR SHALL COORDINATE <u>ALL</u> WORK WITH <u>ALL</u> OTHER TRADES PRIOR TO FABRICATING AND/OR INSTALLING <u>ANY DUCT OR DEVICES</u>. DUCTWORK SHOWN PENETRATING STRUCTURAL WALLS MUST BE COORDINATED WITH THE STRUCTURAL PLANS AND MAY REQUIRE DUCTWORK TRANSITIONS AND/OR OFFSETS NOT SPECIFICALLY SHOWN ON THE DRAWINGS. ANY ADDITIONAL STRUCTURAL OPENINGS SHALL BE BROUGHT TO THE ATTENTION OF THE GENERAL CONTRACTOR PRIOR TO INSTALLATION.
- K. NOT ALL DUCT FITTINGS ARE SHOWN ON PLAN. PROVIDE OFFSET FITTINGS AND ELBOWS AS REQUIRED TO CROSS DUCTWORK AND COORDINATE WITH OTHER TRADES.

SUPPLY AIR D	SUPPLY AIR DEVICE CONNECTION SCHEDULE					
AIRFLOW RANGE	MINIMUM DUCT TAP AND DEVICE NECK SIZE					
(CFM)	SQUARE (IN.)	ROUND(IN.)				
0 - 100	6 X 6	6"Ø				
101 - 200	8 X 8	8"Ø				
201 - 350	10 X 10	10"Ø				
351 - 550	12 X 12	12"Ø				

KEYED NOTES BY SYMBOL '(#)'

- 1. PROVIDE NEW AUTO-CHANGEOVER, 7-DAY PROGRAMMABLE THERMOSTAT. COORDINATE FINAL LOCATIONS WITH OWNER AND ARCHITECT PRIOR TO INSTALLATION.
- 2. PROVIDE HORIZONTAL AIR HANDLING UNIT SUSPENDED FROM STRUCTURE WITH VIBRATION ISOLATORS, SECONDARY DRAIN PAN, AUTOMATIC SHUT OFF FLOAT SWITCH, RETURN AIR FILTER FRAME AND MANUAL BALANCING DAMPER AT OUTSIDE AIR DUCT CONNECTION. ALL DUCTED AIR HANDLING UNITS SHALL BE PROVIDED WITH FIRE ALARM RELAY TO SHUT DOWN AIR HANDLING UNIT UPON ACTIVATION OF THE BUILDING FIRE ALARM SYSTEM. COORDINAT WITH FIRE ALARM CONTRACTOR FOR REQUIREMENTS. COORDINATE EXACT ELEVATION WITH STRUCTURE, DUCTWORK AND PIPING PRIOR TO INSTALLATION. REFER TO MANUFACTURER'S INSTALLATION INSTRUCTIONS FOR REQUIREMENTS. PROVIDE CLEARANCE AND ACCESS TO UNIT AS RECOMMENDED BY THE MANUFACTURER.
- 3. PROVIDE MANUAL BALANCING DAMPER AT EACH TAKEOFF TO SUPPLY AND RETURN AIR
- 4. PROVIDE CEILING CASSETTE AIR HANDLING UNIT, MOUNTED IN LAY-IN CEILING. COORDINATE WITH CEILING GRID INSTALLER PRIOR TO CONSTRUCTION FOR REQUIREMENTS.
- 5. PROVIDE DUCT SMOKE DETECTOR, INTERLOCKED WITH ROOFTOP UNIT TO SHUT DOWN UNIT UPON ACTIVATION. COORDINATE WITH FIRE ALARM CONTRACTOR FOR REQUIREMENTS.
- 6. PROVIDE PACKAGE ROOFTOP UNIT MANUFACTURER'S COMBINATION PROGRAMMABLE WITH FACTORY-MOUNTED CONTROLLER, CAPABLE OF DIRECT TEMPERATURE CONTROL, HUMIDITY CONTROL AND DEMAND-CONTROL VENTILATION.
- 7. PROVIDE MANUFACTURER'S CONTROLLER FOR THE INDICATED MECHANICAL EQUIPMENT. REFER TO MANUFACTURER'S INSTALLATION INSTRUCTIONS, AND COORDINATE WITH ELECTRICAL AND FIRE ALARM CONTRACTORS FOR REQUIREMENTS PRIOR TO CONSTRUCTION.



KEY PLAN



AUSTIN, TX 78745 (512) 956-2971 TBPE FIRM #:F-22791 COPYRIGHT © 2023

THIS DOCUMENT, THE IDEAS AND DESIGNS INCORPORATED HEREIN ARE AND SHALL REMAIN THE PROPERTY OF POWER FORWARD, LLC. THESE DOCUMENTS ARE NOT THE BE USED OR ALTERED, IN WHOLE OR IN PART, FOR ANY PURPOSE OTHER THAN THE ORIGINAL INTENDED USE.

CONSTRUCTION SET

SEAUX+PIERCE architecture

AUSTIN, TX. 78734 P: (512) 419-9301

Renovations and additions:

WWW.SEAUXPIERCE.COM

PROJECT NO. 21-002 MECHANICAL DUCTWORK

PLAN - AREA D

Burnet City Hall 301 E. Jackson Street

Burnet, Texas 78611 DATE REVISED NOTES

CITY PLAN REVIEW #1 VE REVISION ADDENDUM 4

⊘ 2022 · SEAUX+PIERCE architecture



SECTION 230800 - COMMISSIONING OF HVAC

PART 1 GENERAL

1.01 SUMMARY

- A. This section covers the Contractor's responsibilities for commissioning; each subcontractor or installer responsible for the installation of a particular system or equipment item to be commissioned is responsible for the commissioning activities relating to that system or equipment item.
- B. The Commissioning Authority (CA) directs and coordinates all commissioning activities and provides Prefunctional Checklists and Functional Test Procedures for Contractor's use.
- C. The entire HVAC system is to be commissioned, including commissioning activities for the following specific items:
 - 1. Major and minor equipment items.
 - 2. Piping systems and equipment.
 - 3. Ductwork and accessories.
 - 4. Other equipment and systems explicitly identified elsewhere in Contract Documents as requiring commissioning.
- D. The Prefunctional Checklist and Functional Test requirements specified in this section are in addition to, not a substitute for, inspection or testing specified in other sections.

1.02 REFERENCE STANDARDS

A. ASHRAE Guideline 1.1 - HVAC&R Technical Requirements for the Commissioning Process 2007, with Errata (2012).

1.03 SUBMITTALS

- A. Updated Submittals: Keep the Commissioning Authority informed of all changes to control system documentation made during programming and setup; revise and resubmit when substantial changes are made.
- B. Startup Reports, Prefunctional Checklists, and Trend Logs: Submit for approval of Commissioning Authority.
- C. Draft Training Plan: In addition to requirements specified in Division 1, include:
 - 1. Follow the recommendations of ASHRAE Guideline 1.1.
 - 2. Demonstration and instruction on function and overrides of any local packaged controls.
- D. Training Manuals.

1. Provide three extra copies of the controls training manuals in a separate manual from the O&M manuals.

PART 2 PRODUCTS

2.01 TEST EQUIPMENT

- A. Provide all standard testing equipment required to perform startup and initial checkout and required functional performance testing; unless otherwise noted such testing equipment will NOT become the property of Owner.
- B. Equipment-Specific Tools: Where special testing equipment, tools and instruments are specific to a piece of equipment, are only available from the vendor, and are required in order to accomplish startup or Functional Testing, provide such equipment, tools, and instruments as part of the work at no extra cost to Owner; such equipment, tools, and instruments are to become the property of Owner.

PART 3 EXECUTION

3.01 PREPARATION

- A. Cooperate with the Commissioning Authority in development of the Prefunctional Checklists and Functional Test Procedures.
- B. Furnish additional information requested by the Commissioning Authority.
- C. Prepare a preliminary schedule for HVAC pipe and duct system testing, flushing and cleaning, equipment start-up and testing, adjusting, and balancing start and completion for use by the Commissioning Authority; update the schedule as appropriate.
- D. Notify the Commissioning Authority when pipe and duct system testing, flushing, cleaning, startup of each piece of equipment and testing, adjusting, and balancing will occur; when commissioning activities not yet performed or not yet scheduled will delay construction notify ahead of time and be proactive in seeing that the Commissioning Authority has the scheduling information needed to efficiently execute the commissioning process.
- E. Put all HVAC equipment and systems into operation and continue operation during each working day of testing, adjusting, and balancing and commissioning, as required.

3.02 INSPECTING AND TESTING - GENERAL

- A. Submit startup plans, startup reports, and Prefunctional Checklists for each item of equipment or other assembly to be commissioned.
- B. Perform the Functional Tests directed by the Commissioning Authority for each item of equipment or other assembly to be commissioned.
- C. Provide two-way radios for use during the testing.
- D. Valve/Damper Stroke Setup and Check:

- 1. For all valve/damper actuator positions checked, verify the actual position against the control system readout.
- 2. Set pump/fan to normal operating mode.
- 3. Command valve/damper closed; visually verify that valve/damper is closed and adjust output zero signal as required.
- 4. Command valve/damper open; verify position is full open and adjust output signal as required.
- 5. Command valve/damper to a few intermediate positions.
- 6. If actual valve/damper position does not reasonably correspond, replace actuator or add pilot positioner (for pneumatics).
- E. Deficiencies: Correct deficiencies and re-inspect or re-test, as applicable, at no extra cost to Owner.

3.03 TAB COORDINATION

- A. TAB: Testing, adjusting, and balancing of HVAC.
- B. Coordinate commissioning schedule with TAB schedule.
- C. Have all required Prefunctional Checklists, calibrations, startup and component Functional Tests of the system completed and approved by the Commissioning Authority prior to starting TAB.

3.04 OPERATION AND MAINTENANCE MANUALS

- A. Submit manuals related to items that were commissioned to Commissioning Authority for review; make changes recommended by Commissioning Authority.
- B. Commissioning Authority will add commissioning records to manuals after submission to Owner.

3.05 DEMONSTRATION AND TRAINING

- A. Demonstrate operation and maintenance of HVAC system to Owner' personnel; if during any demonstration, the system fails to perform in accordance with the information included in the O&M manual, stop demonstration, repair or adjust, and repeat demonstration. Demonstrations may be combined with training sessions if appropriate.
- B. These demonstrations are in addition to, and not a substitute for, Prefunctional Checklists and demonstrations to the Commissioning Authority during Functional Testing.
- C. Provide classroom and hands-on training of Owner's designated personnel on operation and maintenance of the HVAC system and all equipment items indicated to be commissioned.

D. Provide the services of manufacturer representatives to assist instructors where necessary.

END OF SECTION

SECTION 260800 - COMMISSIONING OF LIGHTING CONTROLS

PART 1 GENERAL

1.01 SUMMARY

- A. This section covers the Contractor's responsibilities for commissioning; each subcontractor or installer responsible for the installation of a particular system or equipment item to be commissioned is responsible for the commissioning activities relating to that system or equipment item.
- B. The Commissioning Authority (CA) directs and coordinates all commissioning activities and provides System Verification Checklists for Contractor's use.
- C. The entire lighting control system is to be commissioned, including commissioning activities for the following specific items:
 - 1. Emergency ballasts and shunt relays.
 - 2. Manual overrides.
 - 3. Dimming controls.
 - 4. Time schedules.
 - 5. Occupancy sensors.
 - 6. Other equipment and systems explicitly identified elsewhere in Contract Documents as requiring commissioning.
- D. The System Verification Checklist requirements specified in this section are in addition to, not a substitute for, inspection or testing specified in other sections.

1.02 REFERENCE STANDARDS

- A. ASHRAE Guideline 0 The Commissioning Process (2019).
- B. IES DG-29-11 The Commissioning Process Applied to Lighting and Controls Systems (2011).

1.03 SUBMITTALS

- A. Updated Submittals: Keep the Commissioning Authority informed of all changes to control system documentation made during programming and setup; revise and resubmit when substantial changes are made.
- B. System Verification Checklists: Submit for approval of Commissioning Authority.
- C. Draft Training Plan: In addition to requirements specified in Division 1, include:
 - 1. Follow the recommendations of ASHRAE Guideline 0.

2. Demonstration and instruction on function and overrides of any local packaged controls.

D. Training Manuals.

 Provide three extra copies of the controls training manuals in a separate manual from the O&M manuals.

PART 2 PRODUCTS

(NOT USED.)

PART 3 EXECUTION

3.01 PREPARATION

- A. Cooperate with the Commissioning Authority in development of the System Verification Checklists.
- B. Furnish additional information requested by the Commissioning Authority.
- C. Prepare a preliminary schedule for lighting control system testing start and completion for use by the Commissioning Authority; update the schedule as appropriate.
- D. Put all lighting controls systems into operation and continue operation during each working day of commissioning, as required.

3.02 INSPECTING AND TESTING - GENERAL

- A. Submit System Verification Checklists for each item of equipment or other assembly to be commissioned.
- B. Perform the Functional Tests directed by the Commissioning Authority for each item of equipment or other assembly to be commissioned.
- C. Provide two-way radios for use during the testing.
- D. Deficiencies: Correct deficiencies and re-inspect or re-test, as applicable, at no extra cost to Owner.

3.03 OPERATION AND MAINTENANCE MANUALS

- A. Submit manuals related to items that were commissioned to Commissioning Authority for review; make changes recommended by Commissioning Authority.
- B. Commissioning Authority will add commissioning records to manuals after submission to Owner.

3.04 DEMONSTRATION AND TRAINING

A. Demonstrate operation of lighting control system to Owner' personnel; if during any demonstration, the system fails to perform in accordance with the information included

- in the O&M manual, stop demonstration, repair or adjust, and repeat demonstration. Demonstrations may be combined with training sessions if appropriate.
- B. These demonstrations are in addition to, and not a substitute for, System Verification Checklists and demonstrations to the Commissioning Authority during Functional Testing.
- C. Provide the services of manufacturer representatives to assist instructors where necessary.

END OF SECTION